CUSTOMER INFORMATION

CUSTOMER NAME: WHITTEN FAMILY INVESTMENTS
ADDRESS: 2250 COMMERCIAL AVE
PASCO, WA 99302

PROJECT NAME: WAREHOUSE
PROJECT LOCATION: PASCO, WA

GENERAL NOTES

1. MATERIALS

	ASTM DESIGNATION	
STRUCTURAL STEEL PLATE	A529 OR A572 OR A1011SS	GRADE 55
FLANGE MATERIAL	A529	GRADE 55
COLD FORMED LIGHT GAUGE SHAPES	A1011SS	GRADE 55
STRUCTURAL CABLES	A475	GRADE EHS
HOT ROLLED MILL SHAPE	A992	GRADE 50
HOLLOW STRUCTURAL SECTIONS	A500	GRADE B
PBR36 ROOF AND WALL PANELS	A653 OR A792	GRADE 80
STANDING SEAM ROOF	A653 OR A792	GRADE 50
BOLTS	A325	A325
BOLTS	GRADE 5	GRADE 5

2. DESIGN

A. ALL STRUCTURAL STEEL SECTIONS AND WELDED PLATE MEMBER ARE DESIGNED IN ACCORDANCE WITH THE AISC "SPECIFICATIONS FOR THE DESIGN, FABRICATING AND ERECTION OF STRUCTURAL STEEL BUILDING". ALLOWABLE STRESS DESIGN LATEST EDITION.

B. ALL COLD FORMED MEMBERS ARE DESIGNED IN ACCORDANCE WITH AISI "SPECIFICATIONS FOR DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS" LATEST EDITION.

C. ALL WELDING OF STRUCTURAL STEEL IS BASED ON AWSD1.1 "STRUCTURAL WELDING CODE" LATEST EDITION.

3. HIGH STRENGTH BOLT CONNECTIONS:

ALL HIGH STRENGTH BOLTS ARE TYPE ASTM A325 AND ARE TO BE INSTALLED ACCORDING TO THE "SNUG-TIGHT" CONDITIONS AS DEFINED BY THE, RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING A325 OR A490 BOLTS, 2004 EDITION - SECTION 6.1, UNLESS NOTED OTHERWISE.
ALSO, NOTE THAT BOLTS IN STANDARD HOLES DO NOT REQUIRE WASHERS PER THE, RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING A325 OR A490 BOLTS, SECTION 6 (REFERENCE STEEL CONSTRUCTION AISC MANUAL - THIRTEENTH EDITION)

4. A325 BOLT TIGHTENING REQUIREMENTS

ALL HIGH STRENGTH BOLTED CONNECTIONS ARE SUBJECT TO AXIAL TENSION AND OR SLIP CRITICAL. AS SUCH THE BOLTS MUST BE FULLY PRE-TENSIONED AND INSPECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE AISC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 BOLTS AND THE APPLICABLE BUILDING CODE. WASHERS ARE NOT REQUIRED WHEN THE "TURN OF THE NUT" TIGHTENING PROCEDURE IS USED. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ENSURE PROPER BOLT TIGHTNESS.

5. STRUCTURAL PRIMER

ALL STRUCTURAL MEMBERS WILL BE GIVEN ONE COAT OF MANUFACTURER'S STANDARD RUST-INHIBITIVE PRIMER MEETING THE PERFORMANCE REQUIREMENTS OF TT-P-6645. THIS IS NOT A FINISH COAT AND IS NOT INTENDED FOR PROLONGED EXPOSURE TO THE ELEMENTS. REFERENCE AISC 360-05, CODE OF STANDARD PRACTICE SECTION 6.5.1-6.5.4.

6. BUILDER / CONTRACTOR RESPONSIBILITIES

R & M STEEL COMPANY STANDARD PRODUCT SPECIFICATIONS APPLY AND R & M STEEL COMPANY DESIGN, FABRICATION, QUALITY CONTROL STANDARDS AND TOLERANCE WILL GOVERN. IN CASE OF DISCREPANCIES BETWEEN R & M STEEL COMPANY'S PLANS AND PLANS FOR OTHER TRADES R & M STEEL PLANS SHALL GOVERN. (SECTION 3.3 AISC CODE OF STANDARD PRACTICES, LATEST EDITION)

IT IS THE RESPONSIBILITY OF THE BUILDER / CONTRACTOR TO OBTAIN APPROPRIATE APPROVALS AND NECESSARY PERMITS FROM CITY, COUNTY, STATE OR FEDERAL AGENCIES AS REQUIRED.

APPROVAL OF R & M STEEL COMPANY'S DRAWINGS CONSTITUTES THE BUILDER / CONTRACTOR'S ACCEPTANCE OF R & M STEEL COMPANY'S INTERPRETATION OF THE PURCHASE ORDER. (SECTION 4.2.1 AISC CODE OF STANDARD PRACTICES, LATEST EDITION)

THE BUILDER / CONTRACTOR OR A/E FIRM IS RESPONSIBLE FOR THE OVERALL PROJECT. ALL INTERFACE AND COMPATIBILITY CONCERNING ANY MATERIAL NOT FURNISHED BY R & M STEEL COMPANY ARE TO BE CONSIDERED AND COORDINATED BY THE BUILDER / CONTRACTOR OR A/E FIRM UNLESS SPECIFIC DESIGN CRITERIA CONCERNING THIS INTERFACE BETWEEN MATERIALS IS FURNISHED AS PART OF THE PURCHASE ORDER. R & M STEEL COMPANY ASSUMPTIONS WILL GOVERN.

THE BUILDER / CONTRACTORS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL BUILDING COMPONENTS IN ACCORDANCE WITH R & M STEEL COMPANY'S BUILDING "FOR CONSTRUCTION" DRAWINGS. TEMPORARY SUPPORTS OR BRACING REQUIRED FOR THE BUILDING ERECTION WILL BE THE RESPONSIBILITY OF THE ERECTOR TO DETERMINE, FURNISH AND INSTALL. (SECTION 7.9.1 AISC CODE OF STANDARD PRACTICES, LATEST EDITION)

THE DESIGN OF THE ANCHOR BOLT EMBEDDMENT LENGTH IS THE RESPONSIBILITY OF THE FOUNDATION DESIGN ENGINEER. THE LENGTH PROVIDED BY R & M STEEL IS AN ESTIMATED LENGTH AND SHOULD BE ADJUSTED ACCORDING TO THE FOUNDATION DESIGN.

BUILDING LOADS / DESCRIPTION:

WIDTH: 50 LENGTH: 125 HEIGHT: 18 / 18
ROOF PITCH: 1.0:12 / 1.0:12
THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY : IBC 12
THE CONTRACTOR / BUILDER IS TO CONFIRM THAT THESE LOADS COMPLY WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT.

ROOF DEAD LOAD:	2.17	PSF (ROOF PANELS & PURLINS)
COLLATERAL LOAD:	1.0	PSF
ROOF LIVE LOAD:	20.0	PSF
MIN. UNIFORM ROOF SNOW LOAD:	20.0	PSF
GROUND SNOW LOAD: (Pg)	20.0	PSF
ROOF SNOW LOAD: (Pf)	14.0	PSF
BASIC WIND SPEED:	110	MPH
SEISMIC COEFFICIENT:	0.584	

Pf = 0.7 Ce Ct I Pg
Ce = 1
Ct = 1
I = 1
Pg = 20.0 PSF
Pf = 14.0 PSF
UNBALANCED LOAD = 20.8 PSF

IMPORTANCE FACTORS:

WIND LOAD:	1.00
SNOW LOAD:	1.00
SEISMIC:	1.00

ROOF PANELS:

COLOR: Galvalume

WALL PANELS:

COLOR: Sandstone Beige

TRIM COLORS:

GABLE:	Brown
EAVE:	
CORNER:	Brown
DOOR & WINDOW:	Brown
GUTTER:	Brown
DOWNSPOUTS:	Brown
BASE (OPTIONAL):	Brown
8" JAMB/HEAD:	Brown

SOFFIT PANEL:

GABLE EXT:
EAVE EXT:
CANOPY:

LINER PANEL:

LEFT :
RIGHT:
FRONT:
BACK :
ROOF :

EARTHQUAKE DESIGN DATA:
INPUT:

Occupancy Category:	II - Normal
Seismic Importance Factor:	1.00
Mapped Response (Short), Ss:	0.39
Mapped Response (1 sec.), S1:	0.15
Site Class:	D

RESULT:

Seismic Design Category, SDC:	D
Basic Seismic-Force-Resisting Systems:	OCBF, OMF
Analysis Procedure Used:	Equivalent Lateral

FORCE:

Site Coeff (Short), Fa:	1.4856
Site Coeff (1 sec.), Fv:	2.1920
Max. Design Response (Short), Sms:	0.58
Max. Design Response (1 sec.), Sm1:	0.3332
Design Response (Short), Sds:	0.39
Design Response (1 sec.), Sd1:	0.22
Approx. Period (Moment), Ta:	0.2827
Approx. Period (Brace), Ta:	0.1748
Rigid Frame Deflection Limit (Seis):	65
Wind Bent Deflection Limit (Seis):	65

DES_CALC:
Seismic Forces:

Roof Bracing:	Endwall Bracing:	End Plates, Frame:
R = 3.2500	R = 3.2500	R = 3.2500
Rho = 1.3000	Omega = 2.0000	Omega = 3.0000
Cs = 0.12	Cs = 0.12	

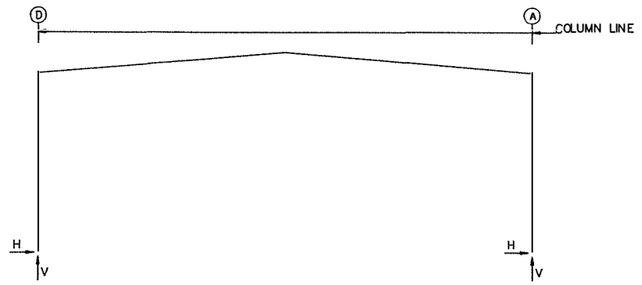
Sidewall Bracing:	Rigid Frames:
R = 3.2500	R = 3.2500
Omega = 2.0000	Rho = 1.3000
Cs = 0.12	Cs = 0.12

Total Base Shear:
Longitudinal Force, V = 4.64 (k)
Transverse Force, V = 4.63 (k)

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P.O. Box 580
Caldwell, Idaho 83606
208-454-1800 Fax 208-454-1801

SCALE:
DATE: 4/7/16
JOB LOCATION: PASCO, WA
REVISION:
DRAWN BY:
DRAWING NUMBER: 1 OF 14

FRAME LINES: 2 3 4 5



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

THE VERTICAL COMPONENT OF BRACING REACTION IS INCLUDED IN VERTICAL REACTIONS.

Frm Line	Col Line	Column Reactions (k) Load ID	Hmax H	V Vmax	Load ID	Hmin H	V Vmin	Anc. Bolt Qty	Dia	Base Plate (in) Width	Length	Thick	Grout (in)
2*	D	5	7.1	15.2	1	−4.3	−6.4	4	0.750	6.000	8.500	0.500	0.0
		5	7.1	15.2	3	−1.8	−8.3						
2*	A	2	4.3	−6.4	5	−7.1	15.2	4	0.750	6.000	8.500	0.500	0.0
		5	−7.1	15.2	4	1.8	−6.5						

2* Frame lines: 2 3 4 5

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
2*	D	0.8	2.1	0.3	0.6	6.0	12.5	4.2	8.8	−7.9	−12.8	−0.4	−7.7
2*	A	−0.8	2.1	−0.3	0.6	−6.0	12.5	−4.2	8.7	0.4	−7.7	7.9	−12.8

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
2*	D	−7.6	−7.6	0.1	−2.5	−3.8	−15.9	0.4	−10.6	−0.6	−0.4	0.6	0.4
2*	A	−0.1	−2.5	7.6	−7.6	−0.4	−7.7	3.8	−12.9	−0.6	0.4	0.6	−0.4

Frame Line	Column Line	Seismic_Long Horiz	Seismic_Long Vert	MIN_SNOW Horiz	MIN_SNOW Vert	F1UNB_SL_L Horiz	F1UNB_SL_L Vert	F1UNB_SL_R Horiz	F1UNB_SL_R Vert
2*	D	−1.5	−3.0	6.0	12.5	3.5	8.7	3.5	4.9
2*	A	−1.5	1.0	−6.0	12.5	−3.5	4.9	−3.5	8.7

2* Frame lines: 2 3 4 5

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind Press Horz
1	D	0.3	0.1	1.4	1.0	0.0	−1.8	0.0	−1.2	0.0	−1.8	0.0	−1.2	−1.3
1	C	0.9	0.2	4.9	3.4	2.1	−8.0	0.0	−1.9	2.1	−8.0	0.0	−1.9	−3.1
1	B	0.9	0.2	4.9	3.4	0.0	−1.9	2.1	−8.0	0.0	−1.9	2.1	−8.0	−3.1
1	A	0.3	0.1	1.4	1.0	0.0	−1.2	0.0	−1.8	0.0	−1.2	0.0	−1.8	−1.3

Frm Line	Col Line	Wind Suct Horz	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seis_Left Horz	Seis_Left Vert	Seis_Right Horz	Seis_Right Vert	MIN_SNOW Horz	MIN_SNOW Vert	E1UNB_SL_L Horz	E1UNB_SL_L Vert
1	D	1.5	0.0	−1.9	0.0	−1.0	0.0	0.0	0.0	0.0	0.0	1.4	0.0	1.0
1	C	3.4	0.0	−5.8	0.3	−4.2	0.7	−0.7	0.0	0.7	0.0	4.9	0.0	4.2
1	B	3.4	0.3	−4.2	0.0	−5.8	0.0	0.7	0.7	−0.7	0.0	4.9	0.0	1.5
1	A	1.5	0.0	−1.0	0.0	−1.9	0.0	0.0	0.0	0.0	0.0	1.4	0.0	0.2

Frm Line	Col Line	E1UNB_SL_R Horz	E1UNB_SL_R Vert
1	D	0.0	0.2
1	C	0.0	1.5
1	B	0.0	4.2
1	A	0.0	1.0

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind Press Horz
6	A	0.3	0.1	1.4	1.0	0.0	−1.8	0.0	−1.2	0.0	−1.8	0.0	−1.2	−1.3
6	B	0.9	0.2	4.9	3.4	2.1	−8.0	0.0	−1.9	2.1	−8.0	0.0	−1.9	−3.1
6	C	0.9	0.2	4.9	3.4	0.0	−1.9	2.1	−8.0	0.0	−1.9	2.1	−8.0	−3.1
6	D	0.3	0.1	1.4	1.0	0.0	−1.2	0.0	−1.8	0.0	−1.2	0.0	−1.8	−1.3

Frm Line	Col Line	Wind Suct Horz	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seis_Left Horz	Seis_Left Vert	Seis_Right Horz	Seis_Right Vert	MIN_SNOW Horz	MIN_SNOW Vert	E2UNB_SL_L Horz	E2UNB_SL_L Vert
6	A	1.5	0.0	−1.9	0.0	−1.0	0.0	0.0	0.0	0.0	0.0	1.4	0.0	1.0
6	B	3.4	0.0	−5.8	0.3	−4.2	0.7	−0.7	0.0	0.7	0.0	4.9	0.0	4.2
6	C	3.4	0.3	−4.2	0.0	−5.8	0.0	0.7	0.7	−0.7	0.0	4.9	0.0	1.5
6	D	1.5	0.0	−1.0	0.0	−1.9	0.0	0.0	0.0	0.0	0.0	1.4	0.0	0.2

Frm Line	Col Line	E2UNB_SL_R Horz	E2UNB_SL_R Vert
6	A	0.0	0.2
6	B	0.0	1.5
6	C	0.0	4.2
6	D	0.0	1.0

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions (k) Load ID	Hmax H	V Vmax	Load ID	Hmin H	V Vmin	Anc. Qty	Bolt Dia	Base Plate (in) Width	Length	Thick	Grout (in)
1	D	6	0.9	−1.0	7	−0.8	−1.0	4	0.750	6.500	6.000	0.250	0.0
		5	0.0	1.8	6	0.9	−1.0						
1	C	8	2.0	−4.3	7	−1.9	−3.0	4	0.750	6.500	6.000	0.250	0.0
		5	0.0	6.0	8	2.0	−4.3						
1	B	9	2.0	−4.3	10	−1.9	−3.0	4	0.750	6.500	6.000	0.250	0.0
		5	0.0	6.0	9	2.0	−4.3						
1	A	11	0.9	−1.0	10	−0.8	−1.0	4	0.750	6.500	6.000	0.250	0.0
		5	0.0	1.8	11	0.9	−1.0						
6	A	6	0.9	−1.0	7	−0.8	−1.0	4	0.750	6.500	6.000	0.250	0.0
		5	0.0	1.8	6	0.9	−1.0						
6	B	8	2.0	−4.3	7	−1.9	−3.0	4	0.750	6.500	6.000	0.250	0.0
		5	0.0	6.0	8	2.0	−4.3						
6	C	9	2.0	−4.3	10	−1.9	−3.0	4	0.750	6.500	6.000	0.250	0.0
		5	0.0	6.0	9	2.0	−4.3						
6	D	11	0.9	−1.0	10	−0.8	−1.0	4	0.750	6.500	6.000	0.250	0.0
		5	0.0	1.8	11	0.9	−1.0						

NOTES FOR REACTIONS

1. All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
2. Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
3. Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
4. Building reactions are based on the following building data:

Width (ft)	= 50.0
Length (ft)	= 125.0
Eave Height (ft)	= 18.0/ 18.0
Roof Slope (rise/12)	= 1.0/ 1.0
Dead Load (psf)	= 2.2
Collateral Load (psf)	= 1.0
Live Load (psf)	= 20.0
Min. Uniform Roof Snow Load (psf)	= 20.0
Ground Snow Load (psf) (Pg)	= 20.0
Roof Snow Load (psf) (Pf)	= 14.0
Wind Speed (mph)	= 110.0
Wind Code	= IBC 12
Exposure	= C
Closed/Open	= C
Importance Wind	= 1.00
Importance Seismic	= 1.00
Seismic Zone	= D
Seismic Caeff (Fa*Ss)	= 0.58

Pf = 0.7 Ce Ct I Pg
Ce = 1
Ct = 1
I = 1
Pg = 20.0 PSF
Pf = 14.0 PSF
UNBALANCED LOAD = 20.8 PSF

5. Loading conditions are:
   1. 0.6Dead+0.6Wind_Left1
   2. 0.6Dead+0.6Wind_Right1
   3. 0.6Dead+0.6Wind_Long1
   4. 0.6Dead+0.6Wind_Long2
   5. Dead+Collateral+MIN_SNOW
   6. 0.6Dead+0.6Wind_Suction+0.6Wind_Long1
   7. 0.6Dead+0.6Wind_Pressure+0.6Wind_Long1
   8. 0.6Dead+0.6Wind_Left2+0.6Wind_Suction
   9. 0.6Dead+0.6Wind_Right2+0.6Wind_Suction
   10. 0.6Dead+0.6Wind_Pressure+0.6Wind_Long2
   11. 0.6Dead+0.6Wind_Suction+0.6Wind_Long2

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Proj (in)
○ 20	Jamb	1/2"	GR36	1.00
⊠ 32	Endwall	3/4"	GR36	2.00
⊠ 32	Frame	3/4"	GR36	2.00

BUILDING BRACING REACTIONS

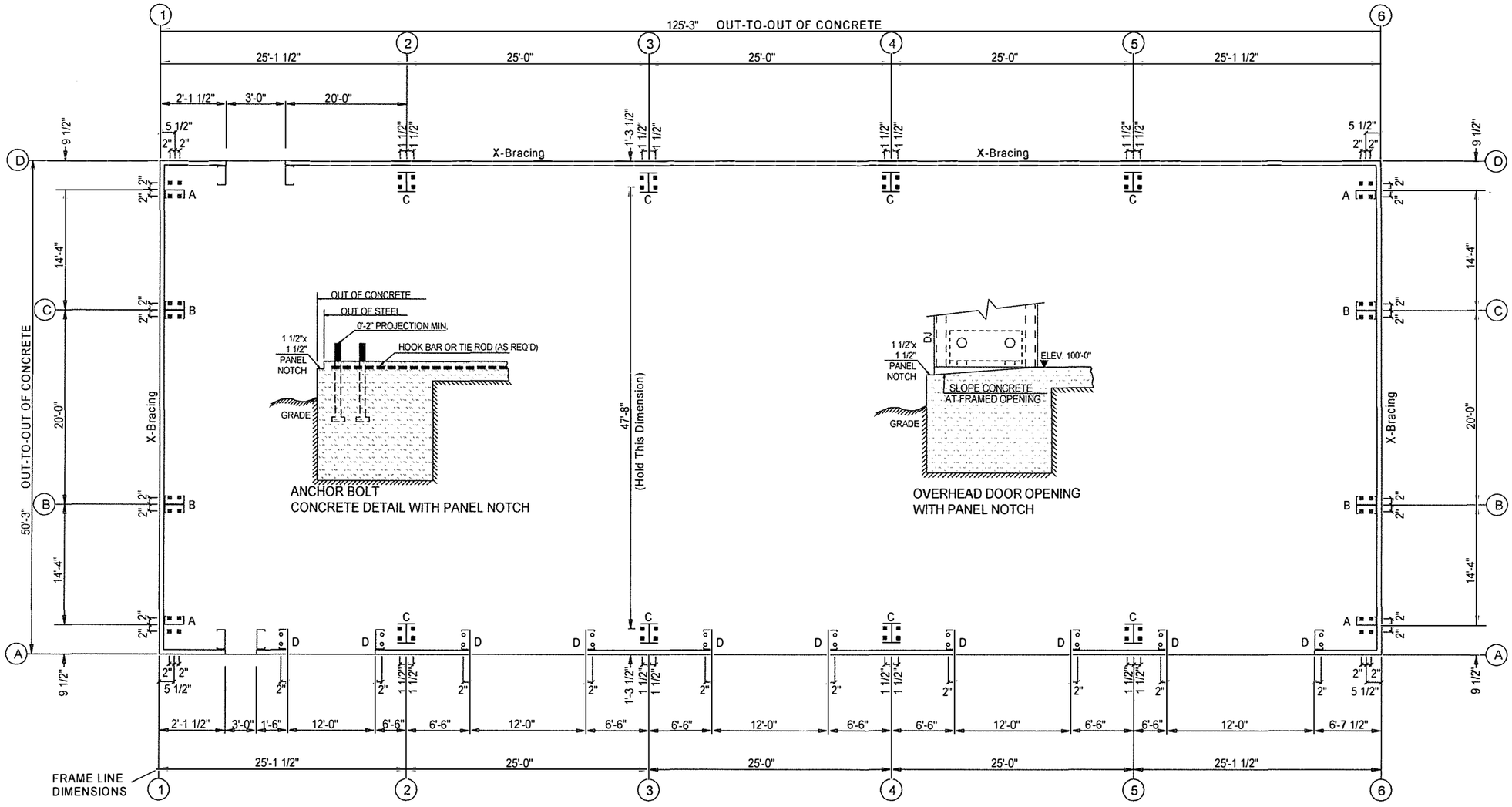
Wall Loc	Wall Line	Col Line	± Reactions (k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel Shear (lb/ft) Wind	Panel Shear (lb/ft) Seis
L_EW	1	C,B	2.1	1.9	0.6	0.5		
F_SW	A	Torsional Bracing Used						
R_EW	6	B,C	2.1	1.9	0.6	0.5		
B_SW	D	5,4	4.6	3.0	2.3	1.5		
		3,2	4.6	3.0	2.3	1.5		

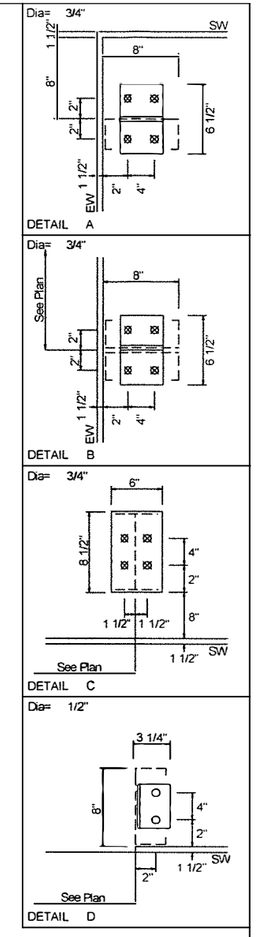
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REVISION

DRAWN BY

DRAWING NUMBER 12 OF 14





ANCHOR BOLT PLAN

NOTE: All Base Plates @ 100'-0" (U.N.)

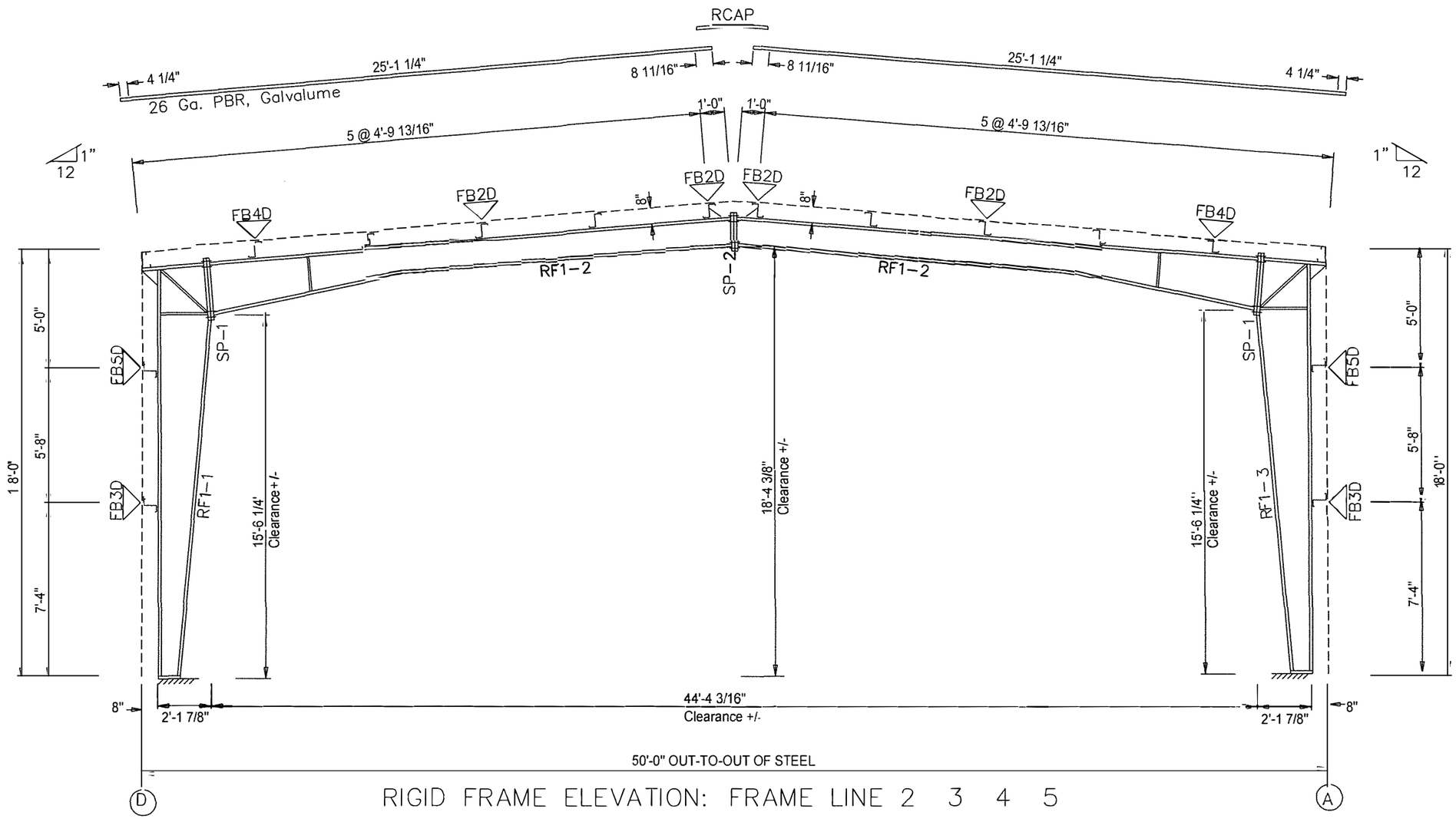
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P.O. Box 580
Caldwell, Idaho 83606
208-454-1800 Fax 208-454-1601

SCALE:
JOB LOCATION: PASCO, WA
DATE: 4/7/16
REVISION:
DRAWN BY:
DRAWING NUMBER: 3 OF 14

SPLICE PLATE & BOLT TABLE									
Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	0	A325	0.750	2.00	6"	1/2"	2'-6 13/16"
SP-2	4	4	0	A325	0.750	2.00	6"	1/2"	1'-6 13/16"

MEMBER TABLE					
Mark	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF1-1	8.0/25.4	0.150	209.8	6 x 1/4" x 207.9 6 x 1/4" x 31.5	6 x 1/4" x 183.3
RF1-2	24.0/12.0 12.0/12.0	0.150 0.150	96.0 172.0	6 x 1/4" x 268.1	6 x 1/4" x 96.7 6 x 1/4" x 171.0
RF1-3	25.4/ 8.0	0.150	209.8	6 x 1/4" x 31.5 6 x 1/4" x 207.9	6 x 1/4" x 183.3

FLANGE BRACES: Both Sides(U.N.)
FBxxD(1)
D - L2x2x14



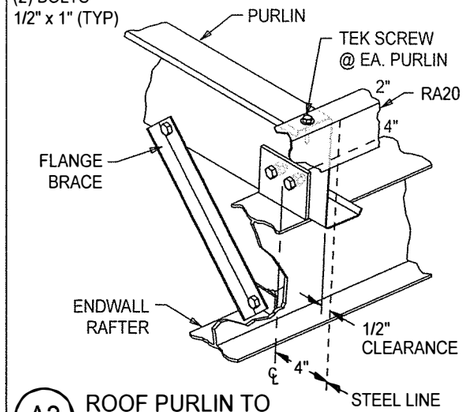
RIGID FRAME ELEVATION: FRAME LINE 2 3 4 5

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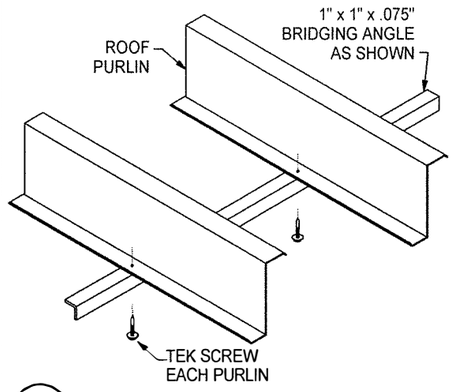
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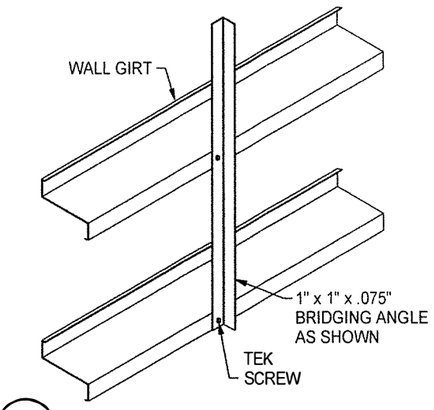
DRAWING NUMBER 4 OF 14



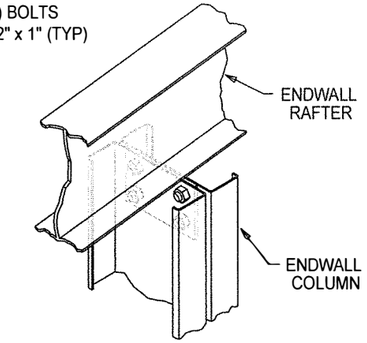
A3 ROOF PURLIN TO ENDWALL RAFTER



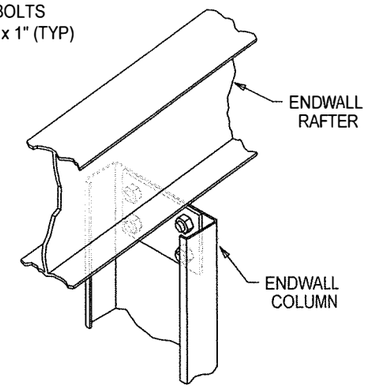
B1 PURLIN BRIDGING



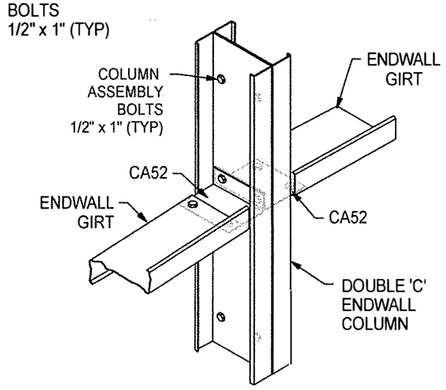
B2 WALL GIRT BRIDGING



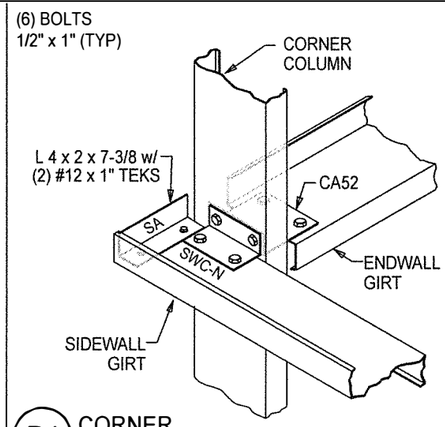
B4 ENDWALL COLUMN TO ENDWALL RAFTER



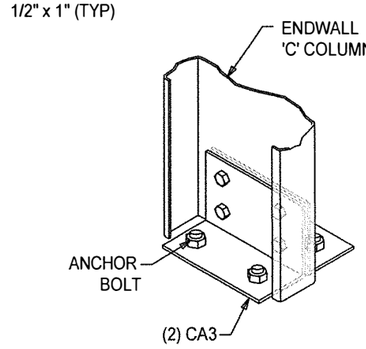
B7 ENDWALL COLUMN TO ENDWALL RAFTER



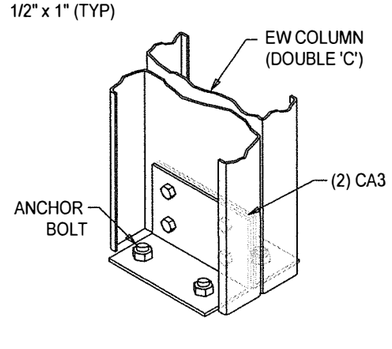
C2 DOUBLE 'C' ENDWALL COLUMN TO GIRTS



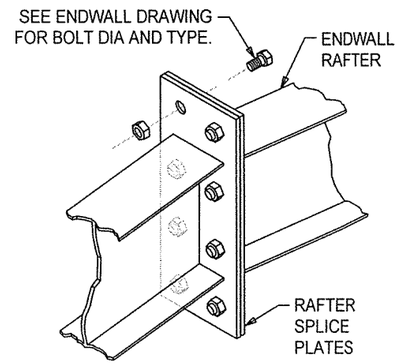
D1 CORNER COLUMN TO WALL GIRTS



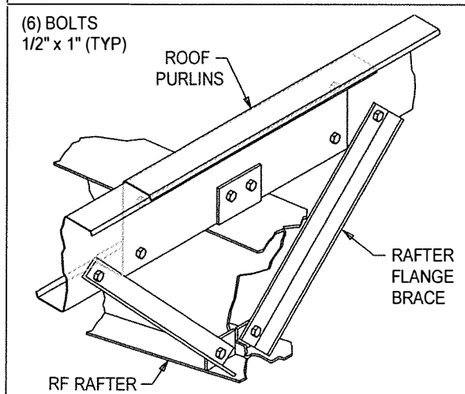
E1 BASE PLATE FOR EW COLUMN



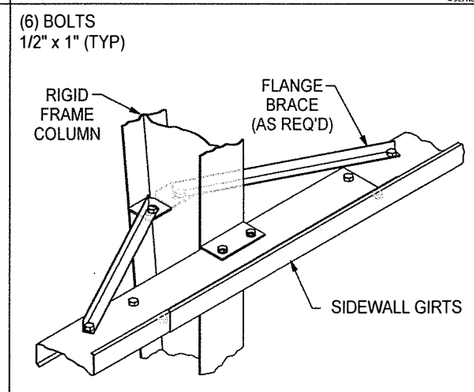
E2 BASE PLATE FOR EW COLUMN



F12 ENDWALL RAFTER SPLICE AT RIDGE



G1 ROOF PURLIN TO INTERIOR RIGID FRAME RAFTER



H1 SIDEWALL GIRT TO INTERIOR FRAME COLUMN

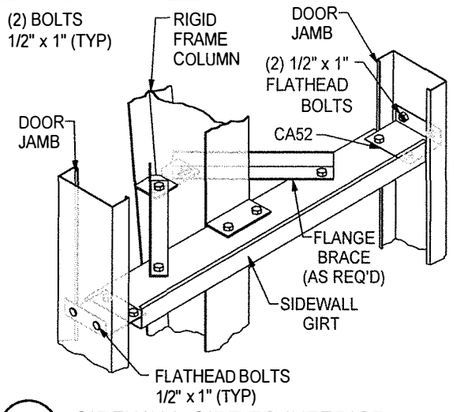
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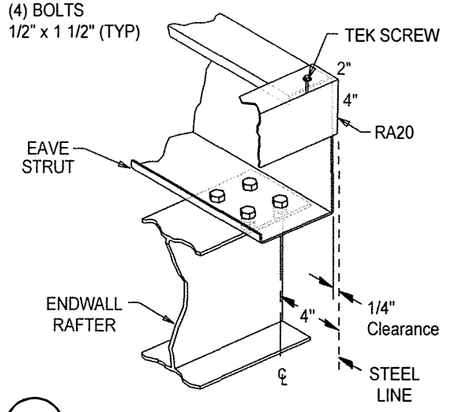
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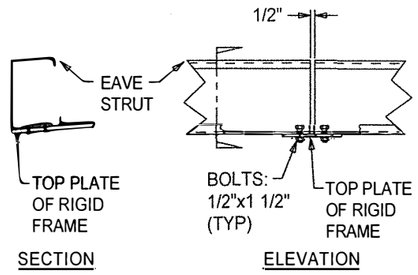
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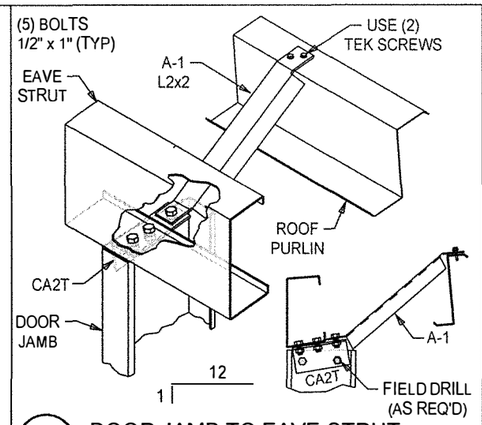
H2 SIDEWALL GIRT TO INTERIOR FRAME COLUMN & DOOR JAMBS



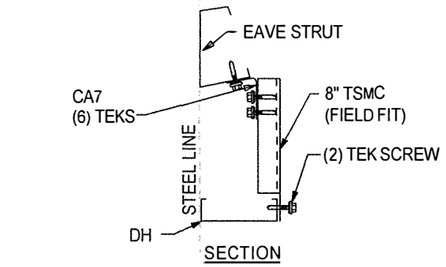
I8 EAVE STRUT TO ENDWALL RAFTER

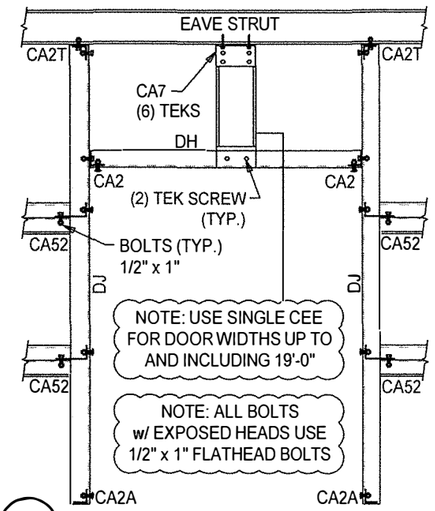


J2 EAVE STRUT TO RIGID FRAME @ INTERIOR BAYS



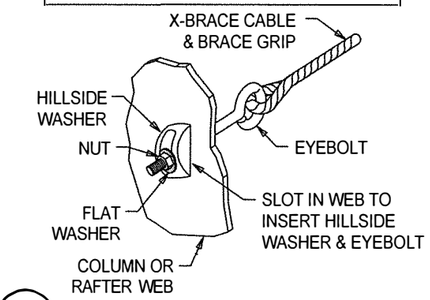
L1 DOOR JAMB TO EAVE STRUT w/ A-1 SUPPORT



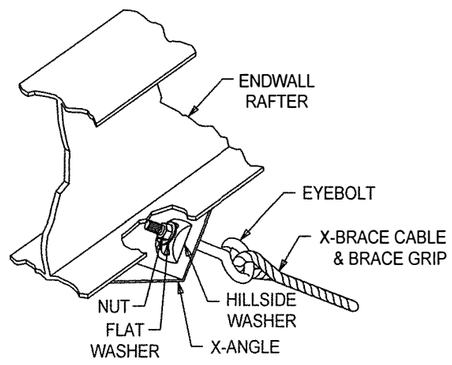


M1 S.S.O.H. DOOR DETAIL

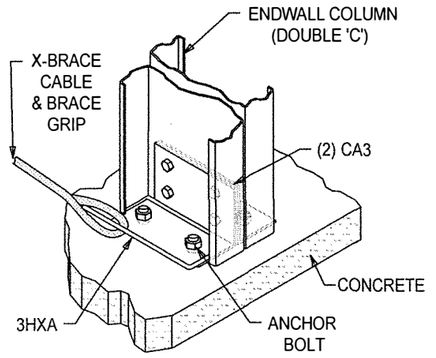
5/8"	E.H.S. CABLE WITH	1 1/4" EYEBOLT
1/2"	E.H.S. CABLE WITH	7/8" EYEBOLT
3/8"	E.H.S. CABLE WITH	3/4" EYEBOLT
5/16"	E.H.S. CABLE WITH	5/8" EYEBOLT
1/4"	E.H.S. CABLE WITH	1/2" EYEBOLT



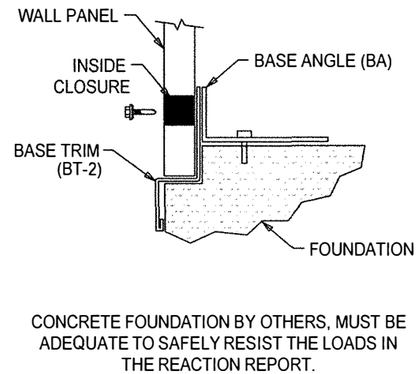
Q2 DIAGONAL CABLE, EYEBOLT END



Q4 DIAGONAL CABLE, EYEBOLT END



Q5 3HXA & (2) CA3 @ ENDWALL COLUMN



CONCRETE FOUNDATION BY OTHERS, MUST BE ADEQUATE TO SAFELY RESIST THE LOADS IN THE REACTION REPORT.

T1 SECTION THRU WALL PANEL AND CONCRETE FOUNDATION

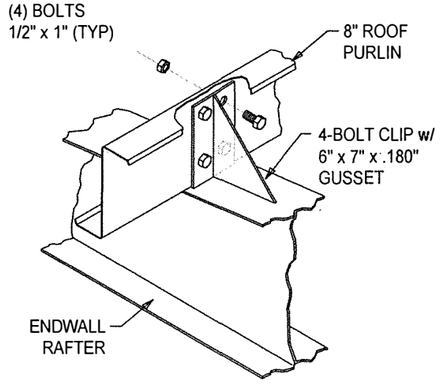
1. FIELD CUT X-BRACE THRU GIRTS (AS REQ'D).
2. FIELD CUT X-BRACE TO LENGTH (AS REQ'D).
3. FIELD FIT AND DRILL GIRTS AT CORNER COLUMN (AS REQ'D).
4. FIELD LOCATE WALK DOORS, WINDOWS, AND LOUVERS (AS REQ'D).
5. FIELD FIT WINDSTRUT (AS REQ'D).
6. FIELD CUT 3'-4" ELEV. GIRT AND TEK TO 8 1/4" WALK DOOR JAMB.
7. FIELD FIT FLANGE BRACES AS REQ'D.
8. LAP BRIDGE ANGLE 1", USE (2) 12-14 x 1" TEKS.
9. LAP RAKE ANGLE 1", USE (2) 12-14 x 1" TEKS.

Y1 FIELD NOTES

FORMAT IS BDDxWTHH

B = WELDED MEMBER TYPE.
DD = MEMBER DEPTH IN INCHES.
W = FLANGE WIDTH IN INCHES. (6=6", 8=8", 0=10", 2=12")
T = FLANGE THICKNESS IN 1/16" INCH UNITS. (3=3/16", 4=1/4", 5=5/16" 6=3/8", 8=1/2", 0=5/8", 2=3/4")
HH = WEB THICKNESS IN DECIMAL INCH UNITS. (14=.1495, 17=.1793, 25=.2500, 31=.3125)

Y2 BUILT-UP NOTES



ARC8 ANTI-ROLL CLIP DETAIL

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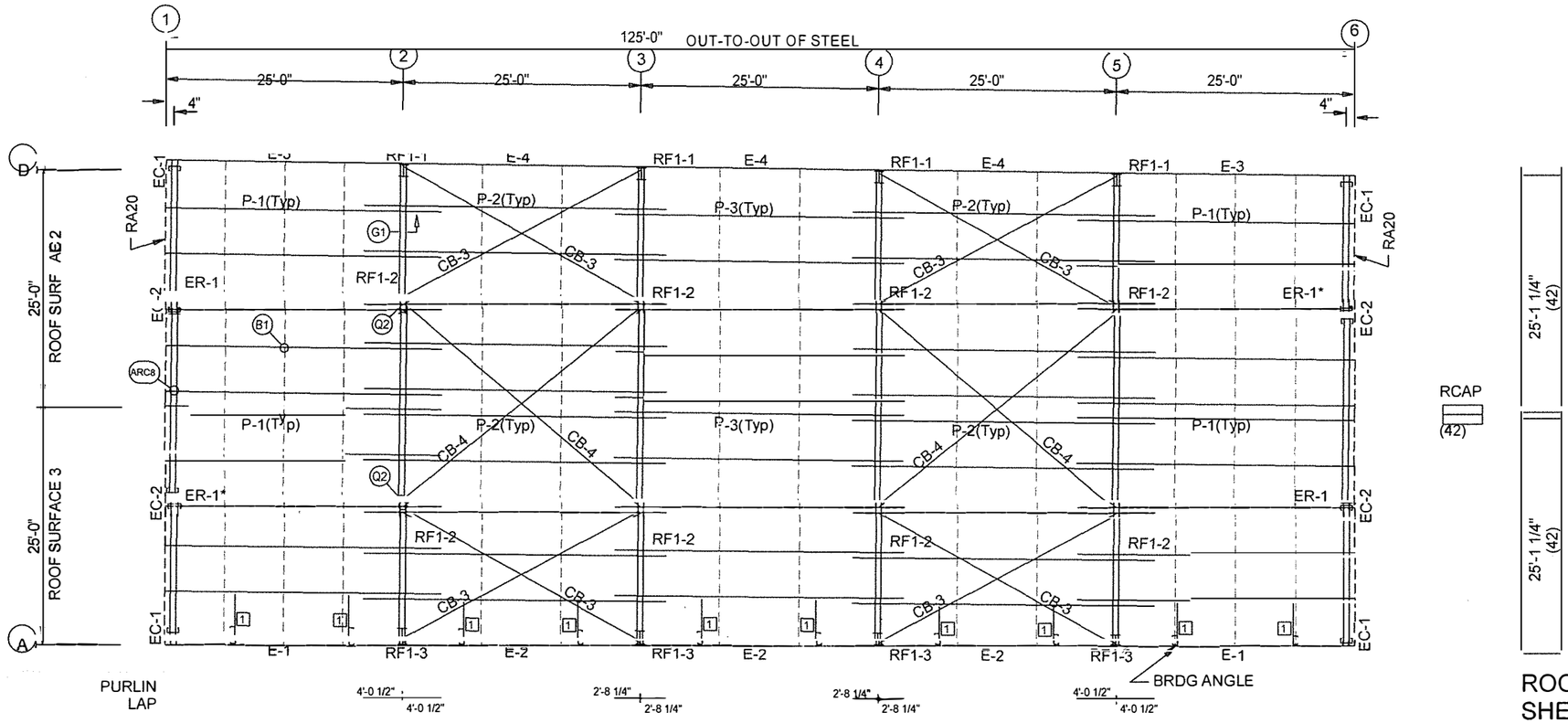
DRAWN BY

DRAWING NUMBER

MEMBER TABLE ROOF PLAN		
MARK	PART	LENGTH
P-1	8x25Z13	29'-0"
P-2	8x25Z16	31'-8 3/4"
P-3	8x25Z16	30'-4 1/2"
E-1	80x5E14	24'-11 1/2"
E-2	80x5E14	24'-11 1/2"
E-3	80x5E14	24'-11 1/2"
E-4	80x5E14	24'-11 1/2"
CB-3	1/4EHS	28'-0"
CB-4	1/4EHS	31'-0"

CONNECTION PLATES ROOF PLAN	
ID	MARK PART
1	A-1

INSTALL ROOF FLANGE BRACE AS PER RIGID FRAME CROSS SECTION



ROOF FRAMING PLAN

RCAP
(42)

25'-1 1/4" (42)

25'-1 1/4" (42)

ROOF SHEETING

PANELS: 26 Ga. PBR
Galvalume

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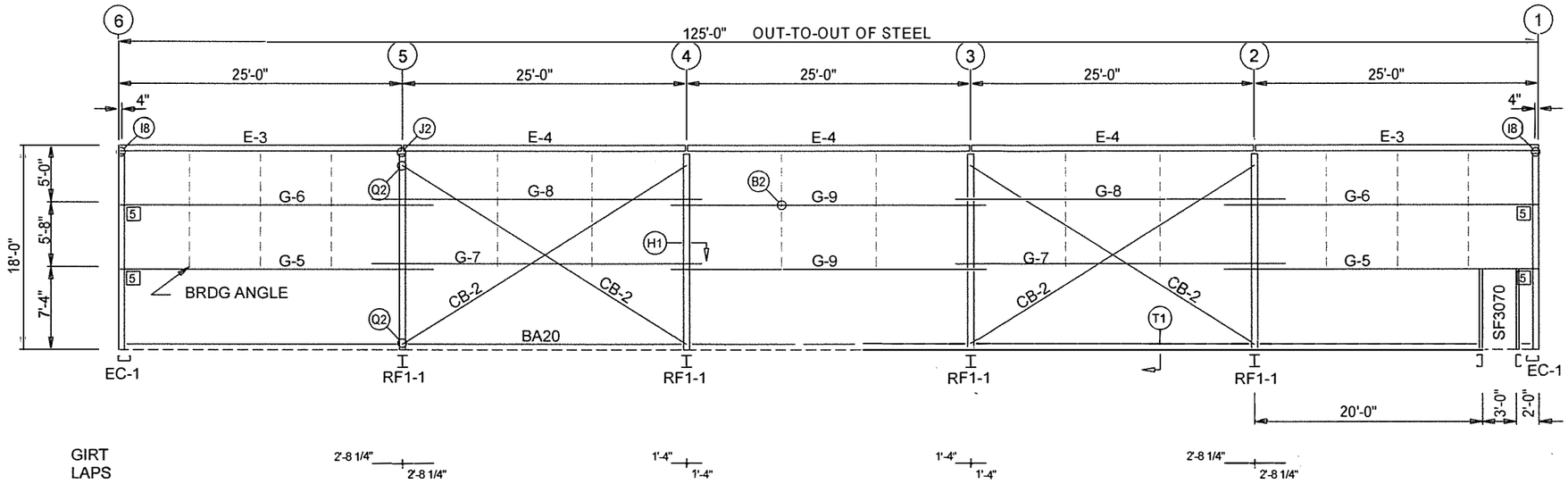
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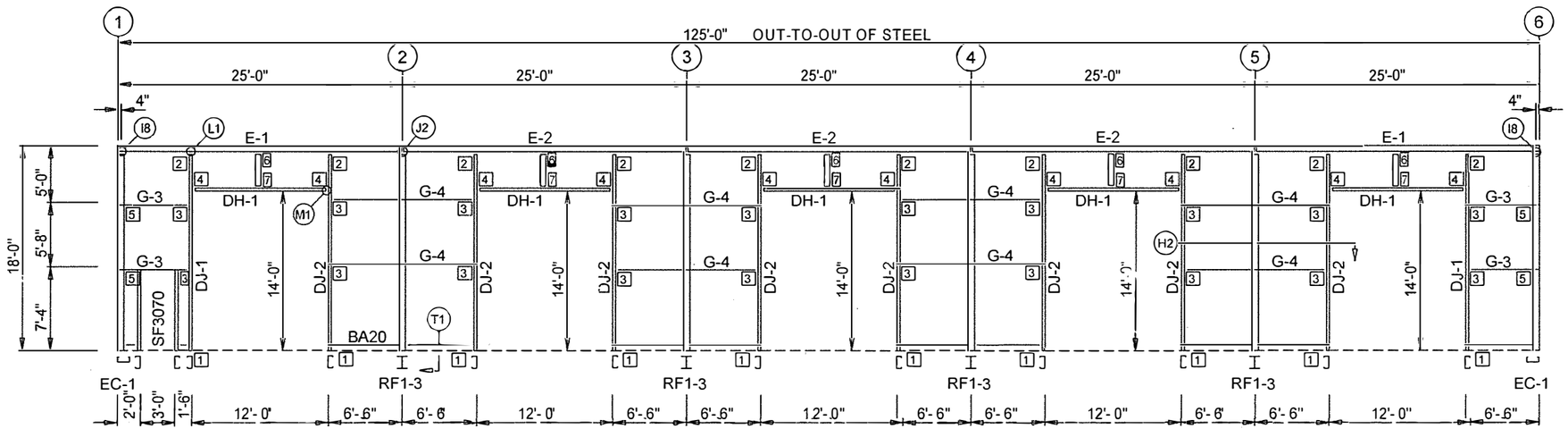
MEMBER TABLE GRID A & D		
MARK	PART	LENGTH
DJ-1	8x25C15	17'-4"
DJ-2	8x25C16	17'-4"
DH-1	8x25C16	12'-0"
E-1	80x5E14	24'-11 1/2"
E-2	80x5E14	24'-11 1/2"
E-3	80x5E14	24'-11 1/2"
E-4	80x5E14	24'-11 1/2"
G-3	8x25Z16	6'-2"
G-4	8x25Z16	12'-4"
G-5	8x25Z15	27'-8 1/4"
G-6	8x25Z16	27'-8 1/4"
G-7	8x25Z15	29'-0 1/4"
G-8	8x25Z16	29'-0 1/4"
G-9	8x25Z16	27'-8"
CB-2	5/16EHS	29'-0"

CONNECTION PLATES GRID A & D	
ID	MARK/PART
1	CA2A
2	CA2T
3	CA52
4	CA2
5	SWC-N
6	CA7
7	TSMC

INSTALL SIDEWALL FLANGE BRACE
AS PER RIGID FRAME CROSS SECTION



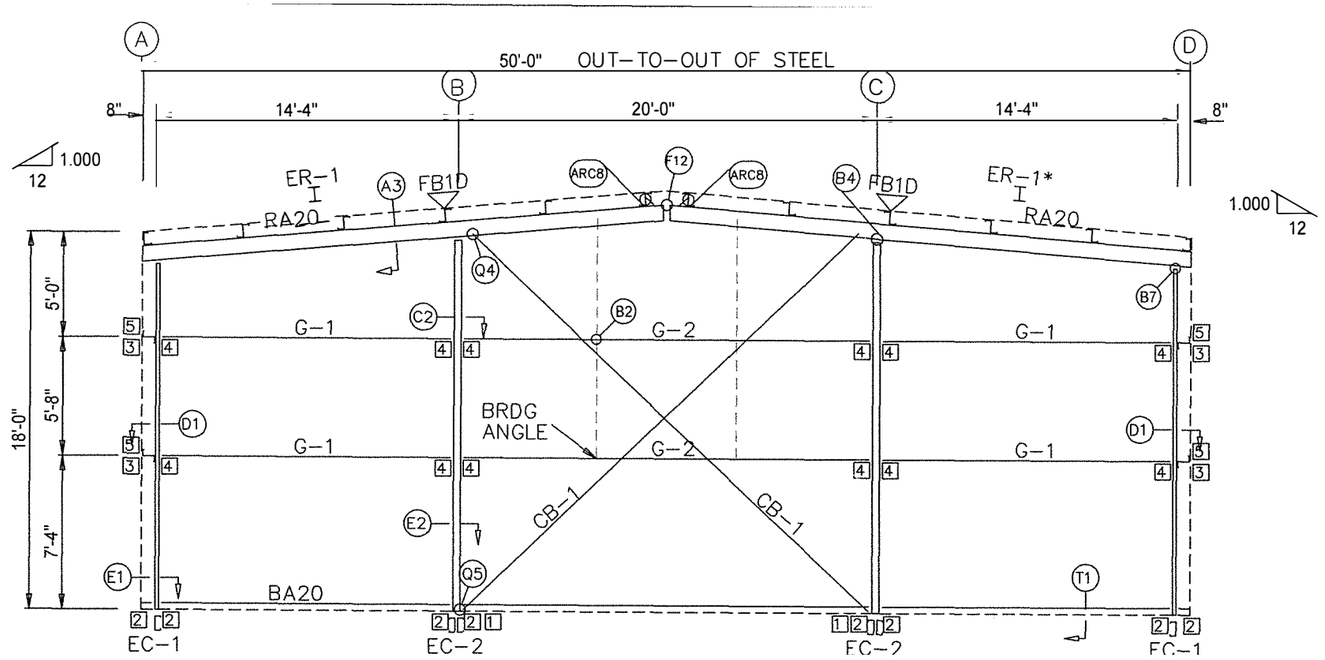
SIDEWALL FRAMING: GRID D



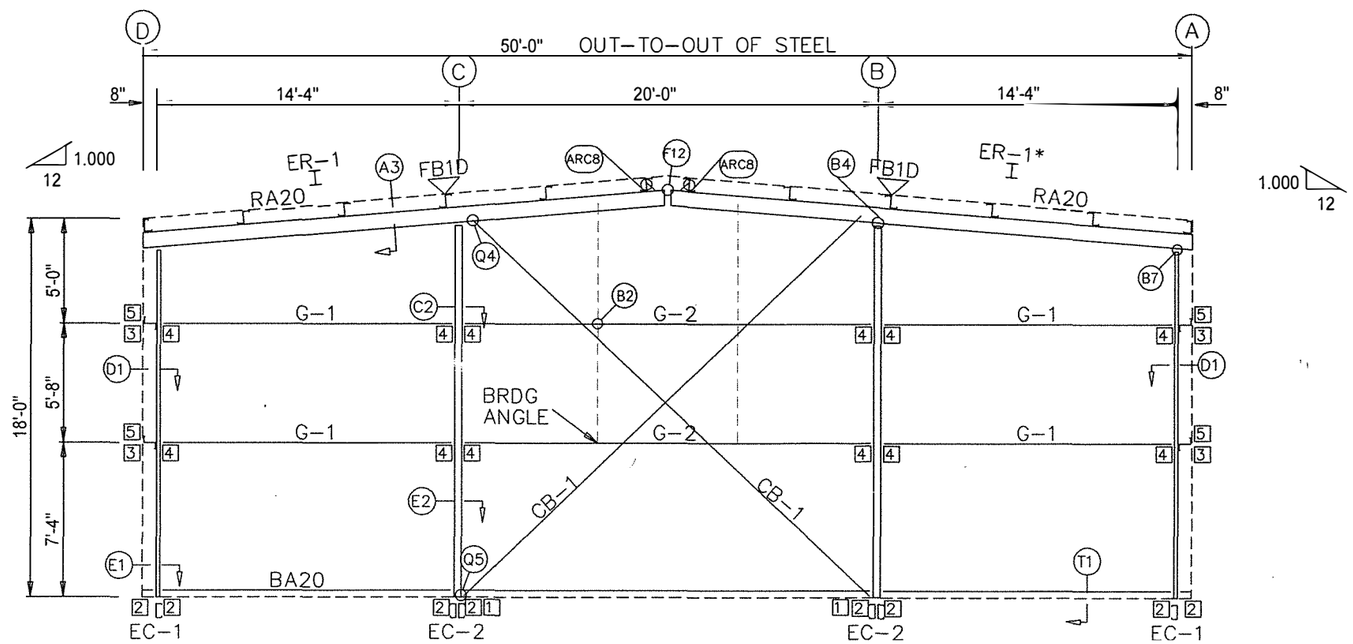
SIDEWALL FRAMING: GRID A

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ENDWALL FRAMING: GRID 6



ENDWALL FRAMING: GRID 1

BOLT TABLE
GRID 1 & 6

LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-1* Columns/Raf	8	A325	5/8"	1 3/4"
	4	GR 5	1/2"	1"

MEMBER TABLE
GRID 1 & 6

MARK	PART	LENGTH
EC-1	8x25C16	16'-8 3/4"
EC-2	D8x5C15	17'-11 1/16"
ER-1	W8x10	25'-1 1/16"
ER-1*	W8x10	25'-1 1/16"
G-1	8x25Z16	13'-8"
G-2	8x25Z15	19'-4"
CB-1	1/4EHS	25'-0"

FLANGE BRACE TABLE
GRID 1 & 6

▽ID	MARK	LENGTH
1	FB1D	1'-4 3/4"

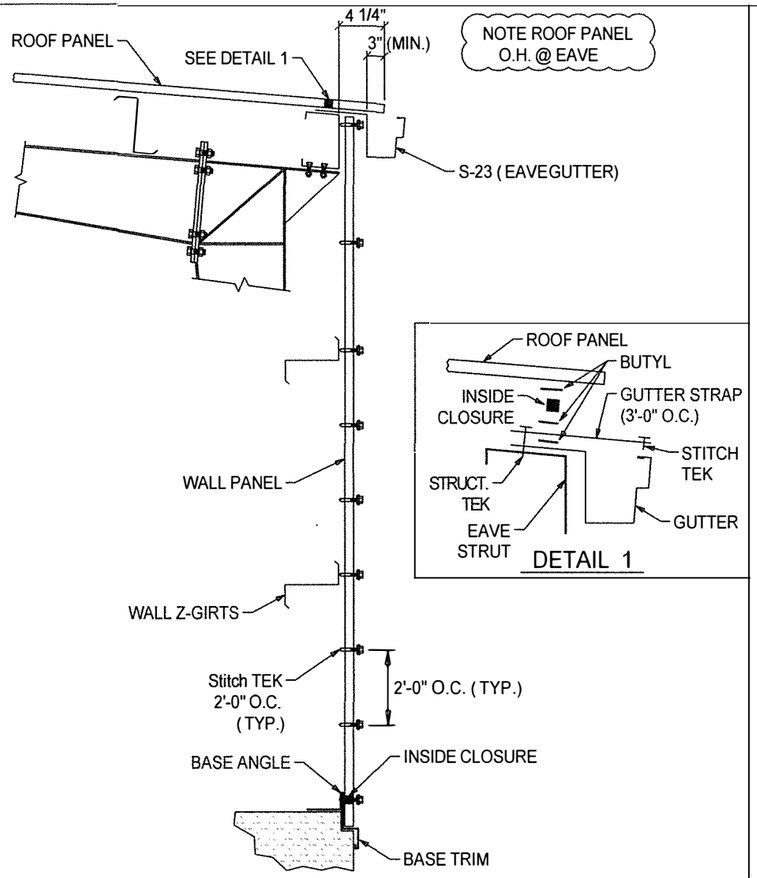
CONNECTION PLATES
GRID 1 & 6

□ID	MARK/PART
1	3HXA
2	CA3
3	SWC-N
4	CA52
5	SA

INSTALL ENDWALL FLANGE BRACE

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ATE:		
		DRAWN BY
		DRAWING NUMBER

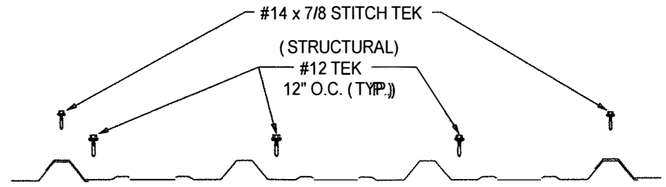


S-23 GUTTER TRIM DETAIL & WALL STITCH TEK LAYOUT

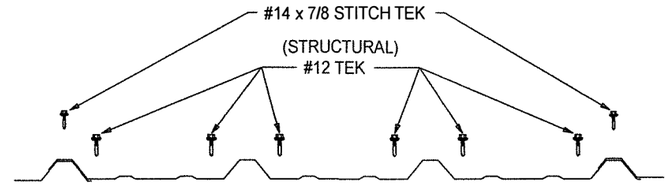
DETAIL 1

TYPICAL TEK SPACING FOR HIGH RIB PANELS

NOTE: ALIGN ROOF PANEL HI-RIBS WITH WALL PANEL HI-RIBS.

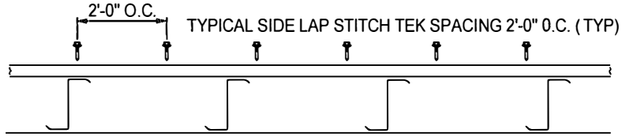


I. TYPICAL SPACING AT INTERMEDIATE PURLINS & GIRTS.

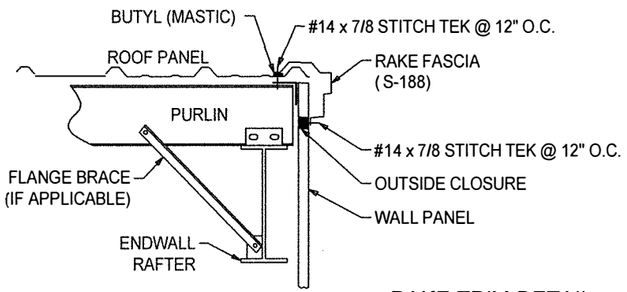


II. TYPICAL SPACING AT PANEL ENDS

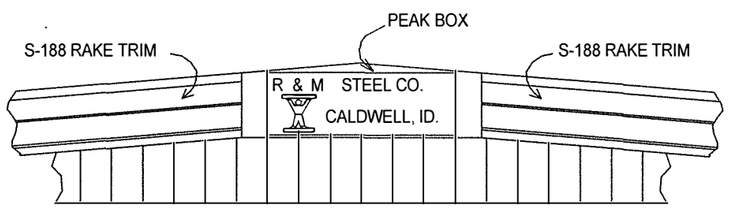
1) TYPICAL SPACING AT EAVE STRUT.
2) TYPICAL SPACING AT RIDGE CAP.
3) TYPICAL SPACING AT BASE ANGLE.
4) TYPICAL SPACING AT RAKE ANGLE.
5) TYPICAL SPACING AT PANEL LAP.



III. TYPICAL STITCH TEK SPACING (SIDE LAP ONLY)

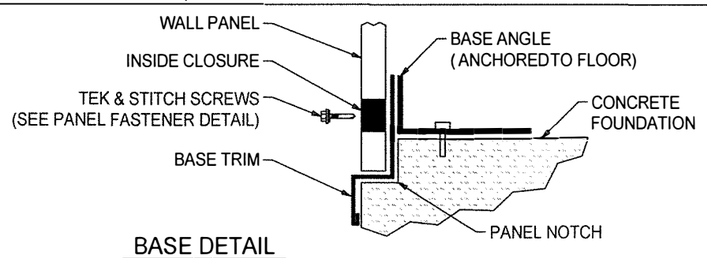


RAKE TRIM DETAIL

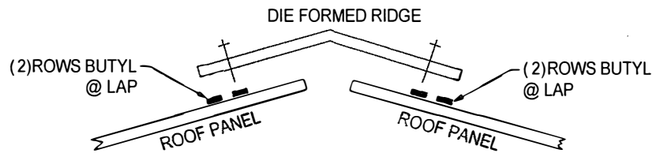


PEAK BOX DETAIL

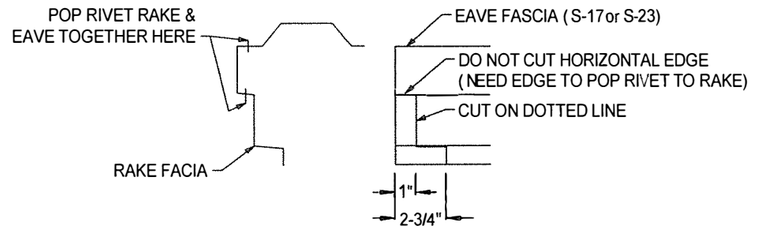
NOTE:
1) MITER S-188 RAKE TRIM @ PEAK & SEAL.
2) INSTALL PEAKBOX & SEAL.
3) MITER S-188 AND S-23 @ CORNERS & SEAL.



BASE DETAIL



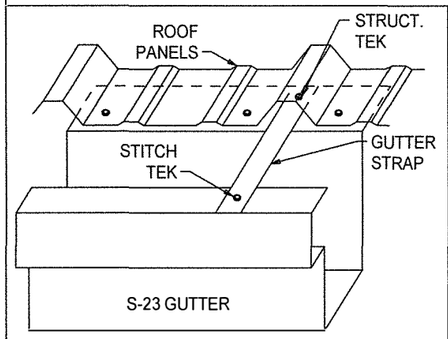
DIE FORMED RIDGE CAP DETAIL



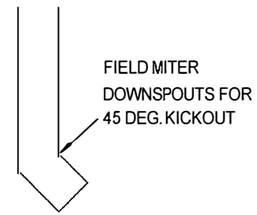
MITER DETAIL: EAVE-TO-RAKE



ROOF PANEL MASTIC DETAIL
@ ROOF PANEL ONLY



GUTTER STRAP DETAIL



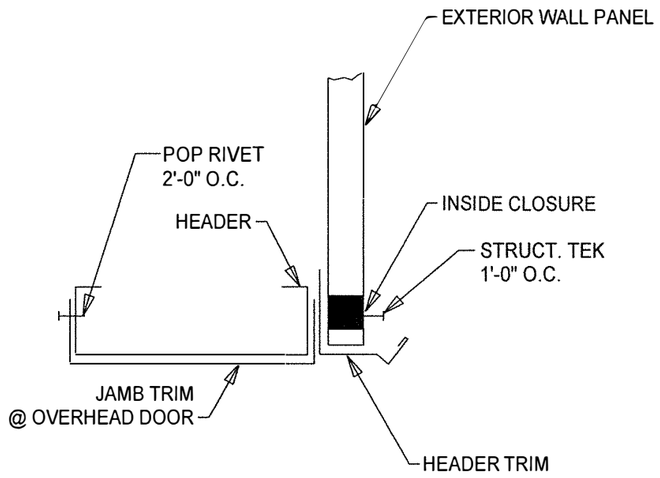
DOWNSPOUT BOTTOM DETAIL

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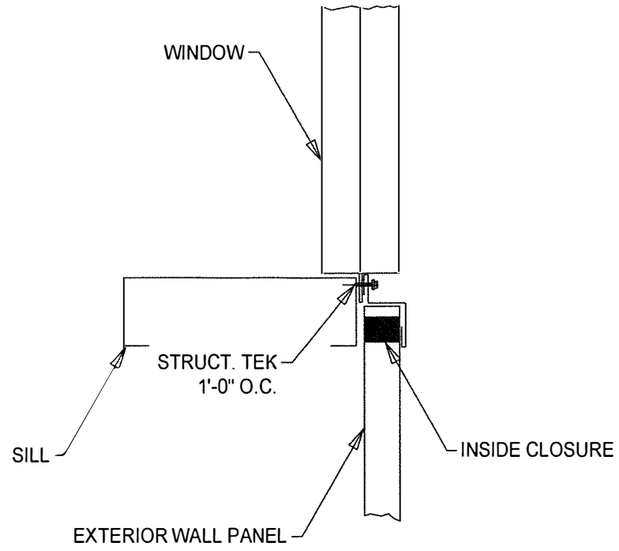
DRAWING NUMBER



NOTE: FIELD NOTCH PANEL AND SLIDE HEADER TRIM BEHIND PANEL AS REQ.

CAULKING (BY OTHERS)

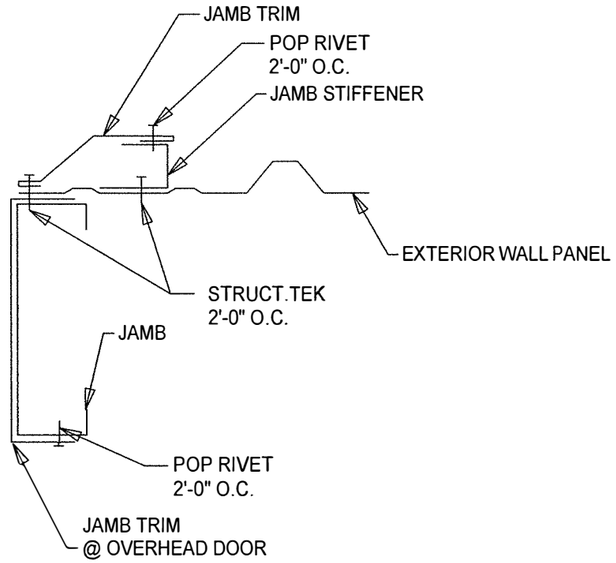
HEADER TRIM DETAIL



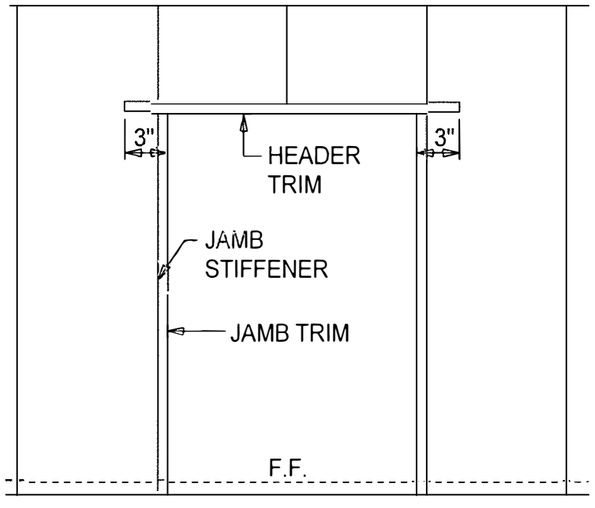
NOTE: FIELD NOTCH PANEL AND SLIDE SILL TRIM BEHIND PANEL AS REQ.

CAULKING (BY OTHERS)

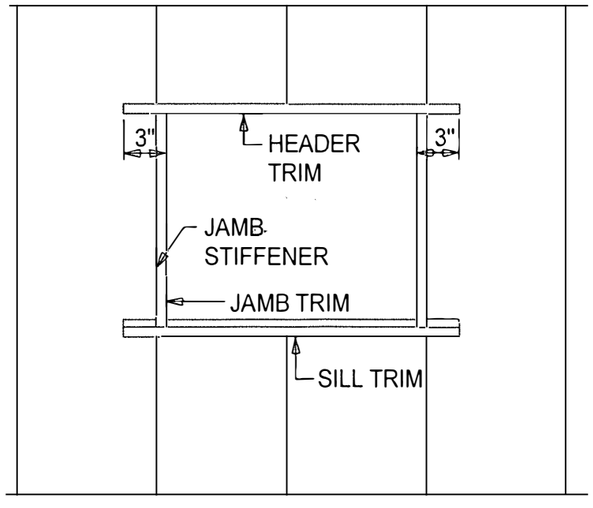
SILL TRIM DETAIL



JAMB TRIM DETAIL



FRAMED OPENING DETAIL FOR OVERHEAD DOOR OR WALK DOOR



FRAMED OPENING DETAIL FOR WINDOW OR LOUVER WITH SILL

R & M STEEL COMPANY
P.O. Box 580
Caldwell, Idaho 83606
208-454-1800 Fax 208-454-1801

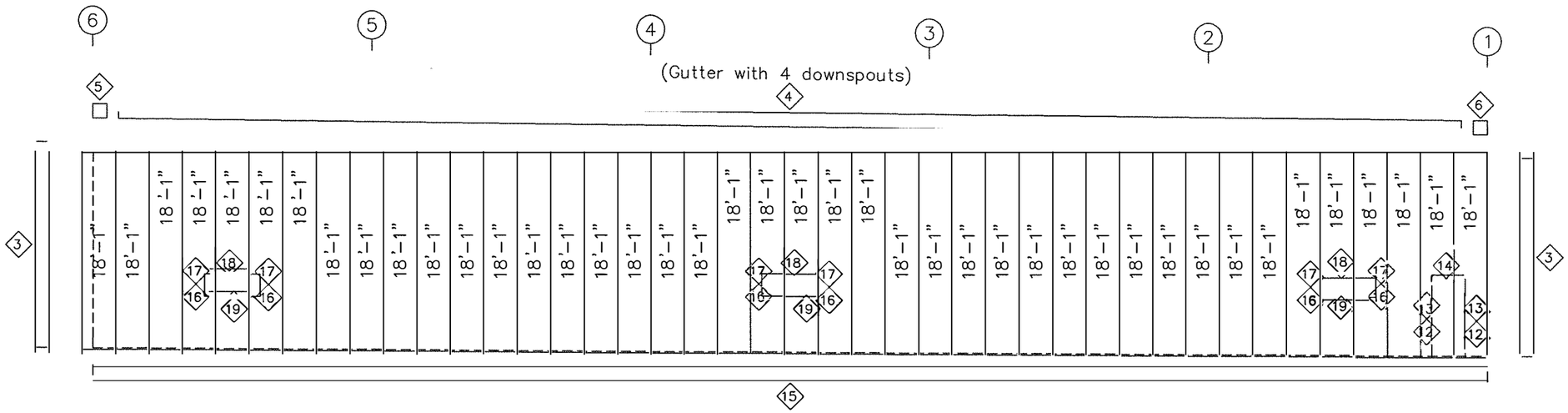
SCALE

REVISION

DRAWN BY

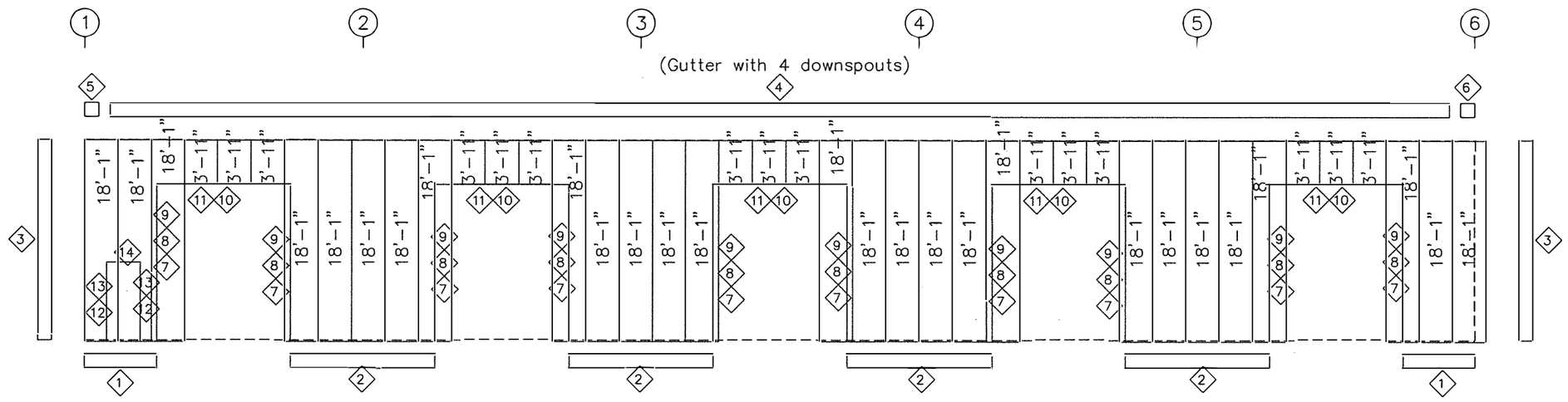
DRAWING NUMBER

TRIM TABLE FRAME LINE		A & D
~~ID~~	~~MARK~~	~~LENGTH~~
~~1~~	~~BT2~~	~~8'-6"~~
2	BT2	15'-0"
3	S-11	18'-0"
4	S-23	32'-0"
5	GECL	1"
6	GECR	1"
7	8-JAMB	14'-0"
8	JMBTRM	14'-6"
9	JMBSTF	14'-6"
10	8-JAMB	12'-0"
11	HEDTRM	13'-0"
12	JMBTRM	7'-4"
13	JMBSTF	7'-4"
14	HEDTRM	4'-0"
15	BT2	32'-0"
16	JMBTRM	2'-1"
17	JMBSTF	2'-1"
18	HEDTRM	6'-0"
19	SILTRM	6'-0"



SIDEWALL SHEETING: GRID D

PANELS: 26 Ga. PBR – Sandstone Beige



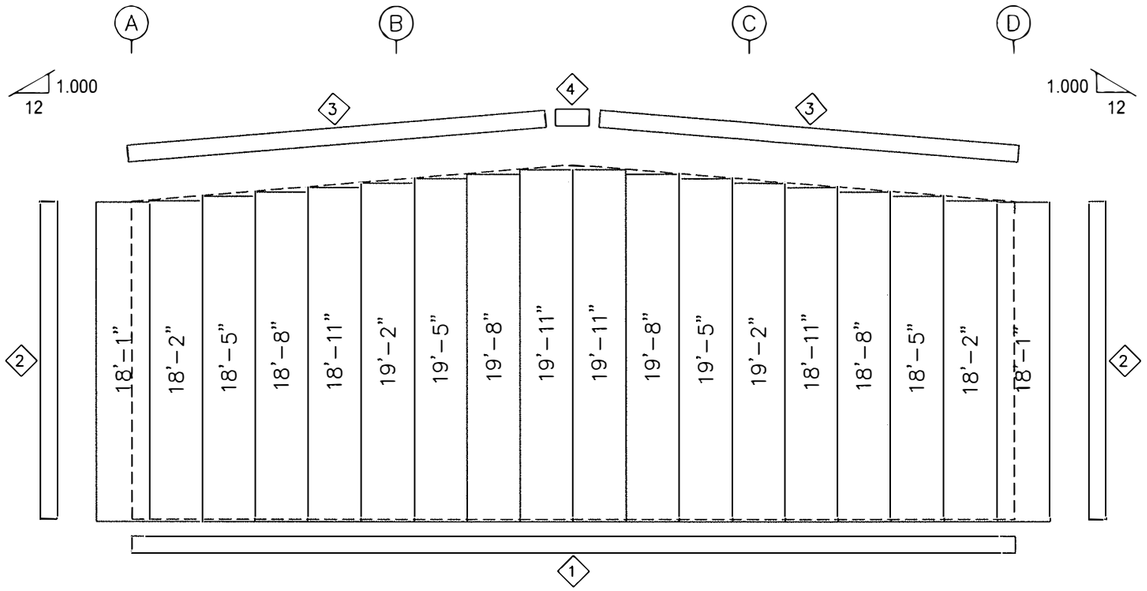
SIDEWALL SHEETING: GRID A

PANELS: 26 Ga. PBR – Sandstone Beige

R & M STEEL COMPANY
P.O. Box 580
Caldwell, Idaho 83606
208-454-1800 Fax 208-454-1881

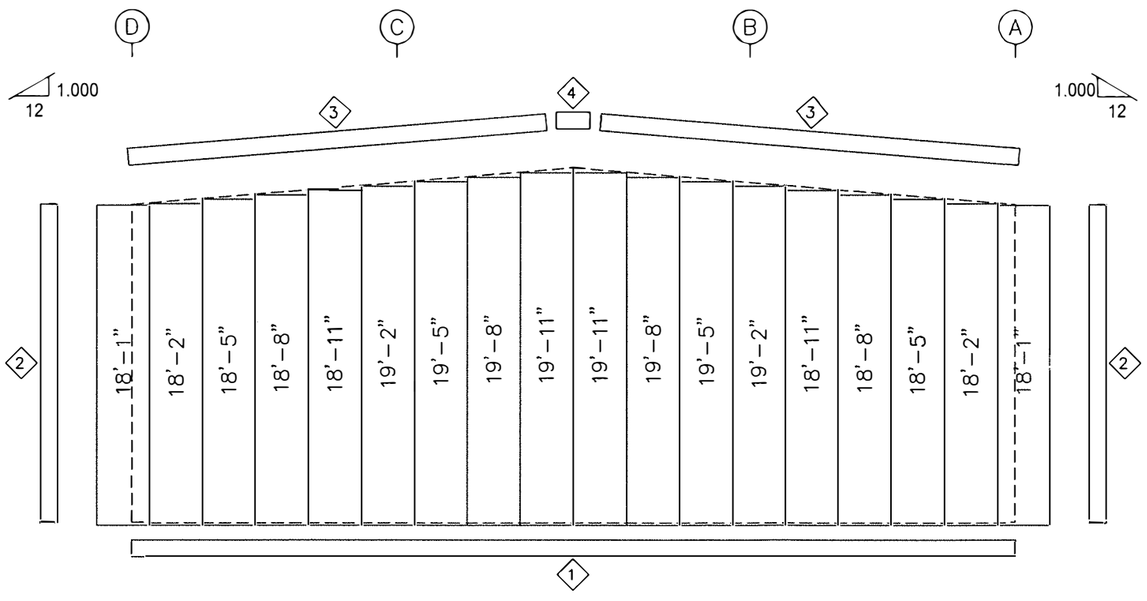
SCALE: | JOB LOCATION | REVISION | DRAWN BY | DRAWING NUMBER 13 OF 14

TRIM TABLE FRAME LINE		1 & 6
◁ID	MARK	LENGTH
1	BT2	26'–6"
2	S–11	18'–0"
3	S–188	26'–6"
4	PK BOX	2'–6"



ENDWALL SHEETING: GRID 6

PANELS: 26 Ga. PBR – Sandstone Beige



ENDWALL SHEETING: GRID 1

PANELS: 26 Ga. PBR – Sandstone Beige

R & M STEEL COMPANY
P.O. Box 580
Caldwell, Idaho 83605
208-454-1800 Fax 208-454-1801

SCALE:

JOB LOCATION

O,

REVISION

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