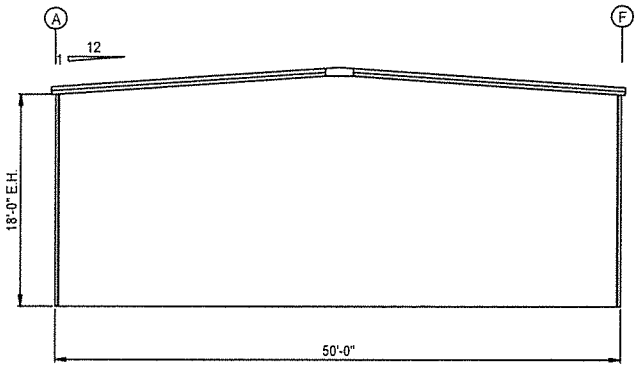
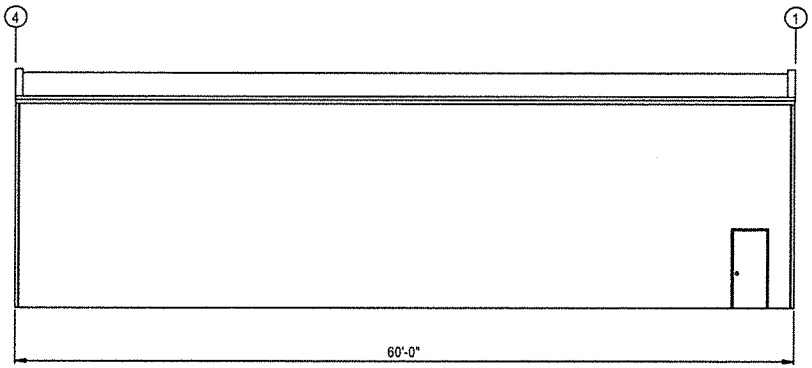
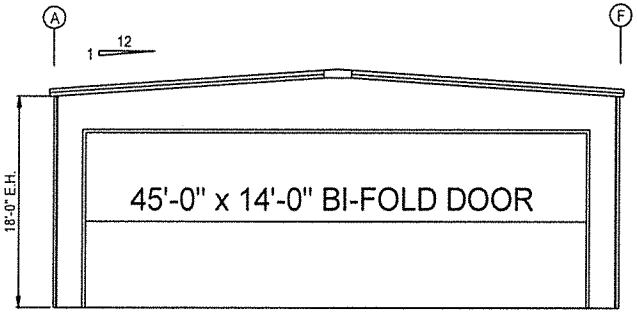


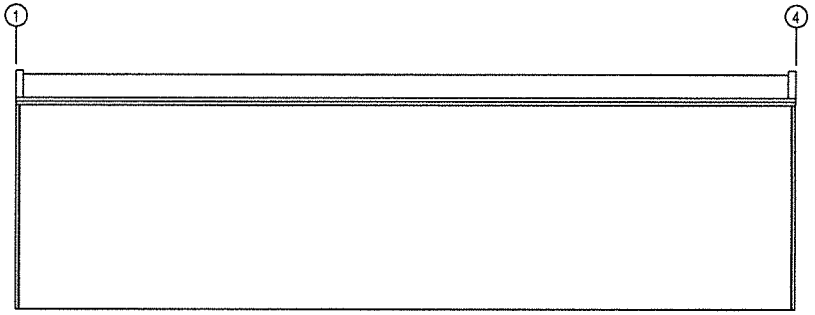
GRID 4 ELEVATION



GRID E ELEVATION



GRID 1 ELEVATION



GRID A ELEVATION

R & M STEEL COMPANY
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SCALE	JOB LOCATION	REVISION
DATE 7/5/16		
E:2016/02/HA050060		DRAWING NUMBER

CUSTOMER INFORMATION

CUSTOMER NAME:
ADDRESS:

PROJECT NAME: HANGAR
PROJECT LOCATION:

GENERAL NOTES

1. MATERIALS

	ASTM DESIGNATION	
STRUCTURAL STEEL PLATE	A529 OR A572 OR A1011SS	GRADE 55
FLANGE MATERIAL	A529	GRADE 55
COLD FORMED LIGHT GAUGE SHAPES	A1011SS	GRADE 55
STRUCTURAL CABLES	A475	GRADE EHS
HOT ROLLED MILL SHAPE	A992	GRADE 50
HOLLOW STRUCTURAL SECTIONS	A500	GRADE B
PBR36 ROOF AND WALL PANELS	A653 OR A792	GRADE 80
STANDING SEAM ROOF	A653 OR A792	GRADE 50
BOLTS	A325	A325
BOLTS	GRADE 5	GRADE 5

2. DESIGN

A. ALL STRUCTURAL STEEL SECTIONS AND WELDED PLATE MEMBER ARE DESIGNED IN ACCORDANCE WITH THE AISC "SPECIFICATIONS FOR THE DESIGN, FABRICATING AND ERECTION OF STRUCTURAL STEEL BUILDING". ALLOWABLE STRESS DESIGN LATEST EDITION.

B. ALL COLD FORMED MEMBERS ARE DESIGNED IN ACCORDANCE WITH AISI "SPECIFICATIONS FOR DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS" LATEST EDITION.

C. ALL WELDING OF STRUCTURAL STEEL IS BASED ON AWSD1.1 "STRUCTURAL WELDING CODE" LATEST EDITION.

3. HIGH STRENGTH BOLT CONNECTIONS:

ALL HIGH STRENGTH BOLTS ARE TYPE ASTM A325 AND ARE TO BE INSTALLED ACCORDING TO THE "SNUG-TIGHT" CONDITIONS AS DEFINED BY THE, RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING A325 OR A490 BOLTS, 2004 EDITION - SECTION 6.1, UNLESS NOTED OTHERWISE.
ALSO, NOTE THAT BOLTS IN STANDARD HOLES DO NOT REQUIRE WASHERS PER THE, RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING A325 OR A490 BOLTS, SECTION 6 (REFERENCE STEEL CONSTRUCTION AISC MANUAL - THIRTEENTH EDITION)

4. A325 BOLT TIGHTENING REQUIREMENTS

ALL HIGH STRENGTH BOLTED CONNECTIONS ARE SUBJECT TO AXIAL TENSION AND OR SLIP CRITICAL. AS SUCH THE BOLTS MUST BE FULLY PRE-TENSIONED AND INSPECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE AISC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 BOLTS AND THE APPLICABLE BUILDING CODE. WASHERS ARE NOT REQUIRED WHEN THE "TURN OF THE NUT" TIGHTENING PROCEDURE IS USED. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ENSURE PROPER BOLT TIGHTNESS.

5. STRUCTURAL PRIMER

ALL STRUCTURAL MEMBERS WILL BE GIVEN ONE COAT OF MANUFACTURER'S STANDARD RUST-INHIBITIVE PRIMER MEETING THE PERFORMANCE REQUIREMENTS OF TT-P-6645. THIS IS NOT A FINISH COAT AND IS NOT INTENDED FOR PROLONGED EXPOSURE TO THE ELEMENTS. REFERENCE AISC 360-05, CODE OF STANDARD PRACTICE SECTION 6.5.1-6.5.4.

6. BUILDER / CONTRACTOR RESPONSIBILITIES

R & M STEEL COMPANY STANDARD PRODUCT SPECIFICATIONS APPLY AND R & M STEEL COMPANY DESIGN, FABRICATION, QUALITY CONTROL STANDARDS AND TOLERANCE WILL GOVERN. IN CASE OF DISCREPANCIES BETWEEN R & M STEEL COMPANY'S PLANS AND PLANS FOR OTHER TRADES R & M STEEL PLANS SHALL GOVERN. (SECTION 3.3 AISC CODE OF STANDARD PRACTICES, LATEST EDITION)

IT IS THE RESPONSIBILITY OF THE BUILDER / CONTRACTOR TO OBTAIN APPROPRIATE APPROVALS AND NECESSARY PERMITS FROM CITY, COUNTY, STATE OR FEDERAL AGENCIES AS REQUIRED.

APPROVAL OF R & M STEEL COMPANY'S DRAWINGS CONSTITUTES THE BUILDER / CONTRACTOR'S ACCEPTANCE OF R & M STEEL COMPANY'S INTERPRETATION OF THE PURCHASE ORDER. (SECTION 4.2.1 AISC CODE OF STANDARD PRACTICES, LATEST EDITION)

THE BUILDER / CONTRACTOR OR A/E FIRM IS RESPONSIBLE FOR THE OVERALL PROJECT. ALL INTERFACE AND COMPATIBILITY CONCERNING ANY MATERIAL NOT FURNISHED BY R & M STEEL COMPANY ARE TO BE CONSIDERED AND COORDINATED BY THE BUILDER / CONTRACTOR OR A/E FIRM UNLESS SPECIFIC DESIGN CRITERIA CONCERNING THIS INTERFACE BETWEEN MATERIALS IS FURNISHED AS PART OF THE PURCHASE ORDER. R & M STEEL COMPANY ASSUMPTIONS WILL GOVERN.

THE BUILDER / CONTRACTORS RESPONSIBLE FOR SETTING OF ANCHOR BOLTS AND ERECTION OF STEEL BUILDING COMPONENTS IN ACCORDANCE WITH R & M STEEL COMPANY'S BUILDING "FOR CONSTRUCTION" DRAWINGS. TEMPORARY SUPPORTS OR BRACING REQUIRED FOR THE BUILDING ERECTION WILL BE THE RESPONSIBILITY OF THE ERECTOR TO DETERMINE, FURNISH AND INSTALL. (SECTION 7.9.1 AISC CODE OF STANDARD PRACTICES, LATEST EDITION)

THE DESIGN OF THE ANCHOR BOLT EMBEDDMENT LENGTH IS THE RESPONSIBILITY OF THE FOUNDATION DESIGN ENGINEER. THE LENGTH PROVIDED BY R & M STEEL IS AN ESTIMATED LENGTH AND SHOULD BE ADJUSTED ACCORDING TO THE FOUNDATION DESIGN.

BUILDING LOADS / DESCRIPTION:

WIDTH: 50 LENGTH: 60 HEIGHT: 18 / 18
ROOF PITCH: 1.0:12 / 1.0:12
THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY : IBC 12

THE CONTRACTOR / BUILDER IS TO CONFIRM THAT THESE LOADS COMPLY WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT.

ROOF DEAD LOAD:	2.28	PSF (ROOF PANELS & PURLINS)
COLLATERAL LOAD:	1.0	PSF
ROOF LIVE LOAD:	20.0	PSF
GROUND SNOW LOAD: (Pg)	42.85	PSF
ROOF SNOW LOAD: (Pf)	30.0	PSF
BASIC WIND SPEED:	115	MPH
SEISMIC COEFFICIENT:	0.875	

Pf = 0.7 Ce Ct I Pg
Ce = 1
Ct = 1
I = 1
Pg = 42.85 PSF
Pf = 30.0 PSF
UNBALANCED LOAD = 40.5 PSF

IMPORTANCE FACTORS:

WIND LOAD:	1.00
SNOW LOAD:	1.00
SEISMIC:	1.00

ROOF PANELS:

COLOR: Sandstone Beige

WALL PANELS:

COLOR: Sandstone Beige

TRIM COLORS:

GABLE:	Brown
EAVE:	Brown
CORNER:	Brown
DOOR & WINDOW:	Brown
GUTTER:	
DOWNSPOUTS:	
BASE (OPTIONAL):	Brown
8" JAMB/HEAD:	Brown

SOFFIT PANEL:

GABLE EXT:
EAVE EXT:
CANOPY:

LINER PANEL:

LEFT :
RIGHT:
FRONT:
BACK :
ROOF :

EARTHQUAKE DESIGN DATA:

INPUT:

Occupancy Category:	II - Normal
Seismic Importance Factor:	1.00
Mapped Response (Short), Ss:	0.71
Mapped Response (1 sec.), S1:	0.21
Site Class:	D

RESULT:

Seismic Design Category, SDC:	D
Basic Seismic-Force-Resisting Systems:	OCBF, OMF
Analysis Procedure Used:	Equivalent Lateral

FORCE:

Site Coeff (Short), Fa:	1.2320
Site Coeff (1 sec.), Fv:	1.9840
Max. Design Response (Short), Sms:	0.87
Max. Design Response (1 sec.), Sm1:	0.4127
Design Response (Short), Sds:	0.58
Design Response (1 sec.), Sd1:	0.28
Approx. Period (Moment), Ta:	0.2827
Approx. Period (Brace), Ta:	0.1748
Rigid Frame Deflection Limit (Seis):	65
Wind Bent Deflection Limit (Seis):	65

DES_CALC:

Seismic Forces:

Roof Bracing:	Endwall Bracing:	End Plates, Frame:
R = 3.2500	R = 3.2500	R = 3.2500
Rho = 1.3000	Omega = 2.0000	Omega = 3.0000
Cs = 0.18	Cs = 0.18	

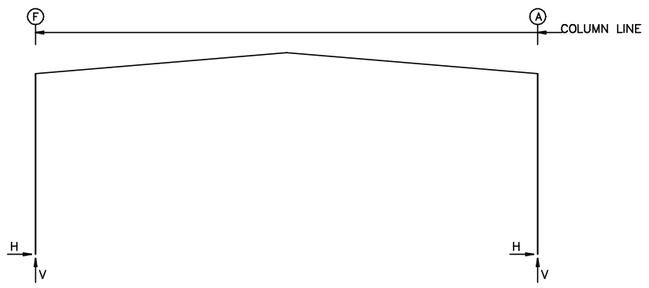
Sidewall Bracing:	Rigid Frames:
R = 3.2500	R = 3.2500
Omega = 2.0000	Rho = 1.3000
Cs = 0.18	Cs = 0.18

Total Base Shear:
Longitudinal Force, V = 3.57 (k)
Transverse Force, V = 3.61 (k)

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Caldwell, Idaho 83606
208-454-1800 Fax 208-454-1801

SCALE:	JOB LOCATION	REVISION
		DRAWN BY
		DRAWING NUMBER
		OF

FRAME LINES: 1 2 3



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions (k) Load ID	Hmax H	V Vmax	Load ID	Hmin H	V Vmin	Anc. Bolt Qty	Bolt Dia	Base Plate (in) Width	Length	Thick	Grout (in)
1	F	1	4.6	11.4	3	−1.1	−1.5	4	0.750	6.000	8.500	0.500	0.0
		7	4.0	11.4									
1	A	4	1.1	−1.5	2	−4.6	11.4	4	0.750	6.000	8.500	0.500	0.0
		8	−4.0	11.4	4	1.1	−1.5						

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

THE VERTICAL COMPONENT OF BRACING REACTION IS INCLUDED IN VERTICAL REACTIONS.

Frm Line	Col Line	Column Reactions (k) Load ID	Hmax H	V Vmax	Load ID	Hmin H	V Vmin	Anc. Bolt Qty	Bolt Dia	Base Plate (in) Width	Length	Thick	Grout (in)
2*	F	2	7.0	17.4	3	−3.3	−5.5	4	0.750	6.000	8.500	0.500	0.0
					5	−0.1	−7.2						
2*	A	4	3.3	−5.5	2	−7.0	17.4	4	0.750	6.000	8.500	0.500	0.0
		2	−7.0	17.4	6	0.1	−7.2						

2* Frame lines: 2 3

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
1	F	1.3	3.3	0.1	0.3	2.1	5.2	3.2	7.8	−3.2	−5.8	0.0	−3.6
1	A	−1.3	3.3	−0.1	0.3	−2.1	5.2	−3.2	7.8	0.0	−3.6	3.2	−5.8

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
1	F	−3.1	−3.4	0.1	−1.2	−0.4	−5.2	−0.6	−4.2	−0.5	−0.3	0.5	0.3
1	A	−0.1	−1.2	3.1	−3.4	0.6	−4.2	0.4	−5.2	−0.5	0.3	0.5	−0.3

Frame Line	Column Line	MIN_SNOW Horiz	MIN_SNOW Vert	F1UNB_SL_L Horiz	F1UNB_SL_L Vert	F1UNB_SL_R Horiz	F1UNB_SL_R Vert
1	F	2.1	5.2	2.6	7.8	2.6	4.3
1	A	−2.1	5.2	−2.6	4.3	−2.6	7.8

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert
2*	F	0.7	1.9	0.2	0.5	4.1	10.0	6.1	15.0	−6.1	−11.1	−0.1	−6.9
2*	A	−0.7	1.9	−0.2	0.5	−4.1	10.0	−6.1	15.0	0.1	−6.9	6.1	−11.1

Frame Line	Column Line	Wind_Left2 Horiz	Wind_Left2 Vert	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
2*	F	−5.8	−6.5	0.2	−2.3	−0.8	−14.0	−1.2	−12.1	−0.7	−0.5	0.7	0.5
2*	A	−0.2	−2.3	5.8	−6.5	1.2	−12.1	0.8	−14.0	−0.7	0.5	0.7	−0.5

Frame Line	Column Line	Seismic_Long Horiz	Seismic_Long Vert	MIN_SNOW Horiz	MIN_SNOW Vert	F2UNB_SL_L Horiz	F2UNB_SL_L Vert	F2UNB_SL_R Horiz	F2UNB_SL_R Vert
2*	F	0.0	−1.9	4.1	10.0	5.0	15.0	5.0	8.3
2*	A	0.0	−1.9	−4.1	10.0	−5.0	8.3	−5.0	15.0

2* Frame lines: 2 3

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Horz	Dead Vert	Wind Press Horz	Wind Suct Horz
1	E	−0.4	0.6	−4.5	4.9
1	B	−0.4	0.6	−4.5	4.9

Frm Line	Col Line	Dead Vert	Collat Vert	Live Vert	Snow Vert	Wind_Left1 Horz	Wind_Left1 Vert	Wind_Right1 Horz	Wind_Right1 Vert	Wind_Left2 Horz	Wind_Left2 Vert	Wind_Right2 Horz	Wind_Right2 Vert	Wind Press Horz
4	A	0.2	0.1	1.1	1.7	0.0	−1.6	0.0	−1.0	0.0	−1.6	0.0	−1.0	−1.4
4	C	0.8	0.2	3.9	5.9	1.9	−7.3	0.0	−1.7	1.9	−7.3	0.0	−1.7	−3.4
4	D	0.8	0.2	3.9	5.9	0.0	−1.7	1.9	−7.3	0.0	−1.7	1.9	−7.3	−3.4
4	F	0.2	0.1	1.1	1.7	0.0	−1.0	0.0	−1.6	0.0	−1.0	0.0	−1.6	−1.4

Frm Line	Col Line	Wind Suct Horz	Wind_Long1 Horz	Wind_Long1 Vert	Wind_Long2 Horz	Wind_Long2 Vert	Seis_Left Horz	Seis_Left Vert	Seis_Right Horz	Seis_Right Vert	MIN_SNOW Horz	MIN_SNOW Vert	E2UNB_SL_L Horz	E2UNB_SL_L Vert
4	A	1.6	0.0	−1.7	0.0	−0.9	0.0	0.0	0.0	0.0	0.0	1.1	0.0	1.8
4	C	3.7	0.0	−5.2	0.3	−3.8	0.9	−0.9	0.0	0.8	0.0	3.9	0.0	7.1
4	D	3.7	0.3	−3.8	0.0	−5.2	0.0	0.8	0.9	−0.9	0.0	3.9	0.0	2.5
4	F	1.6	0.0	−0.9	0.0	−1.7	0.0	0.0	0.0	0.0	0.0	1.1	0.0	0.4

Frm Line	Col Line	E2UNB_SL_R Horz	E2UNB_SL_R Vert
4	A	0.0	0.4
4	C	0.0	2.5
4	D	0.0	7.1
4	F	0.0	1.8

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions (k) Load ID	Hmax H	V Vmax	Load ID	Hmin H	V Vmin	Anc. Qty	Bolt Dia	Base Plate (in) Width	Length	Thick	Grout (in)
1	E	9	2.9	0.4	10	−2.7	0.4	4	0.750	6.000	6.250	0.375	0.0
		11	−0.4	0.7									
1	B	9	2.9	0.4	10	−2.7	0.4	4	0.750	6.000	6.250	0.375	0.0
		11	−0.4	0.7									
4	A	12	1.0	−0.9	13	−0.9	−0.9	4	0.750	6.500	6.000	0.250	0.0
		14	0.0	2.1	12	1.0	−0.9						
4	C	15	2.2	−3.9	13	−2.0	−2.7	4	0.750	6.500	6.000	0.250	0.0
		14	0.0	8.1	15	2.2	−3.9						
4	D	9	2.2	−3.9	10	−2.0	−2.7	4	0.750	6.500	6.000	0.250	0.0
		16	0.0	8.1	9	2.2	−3.9						
4	F	17	1.0	−0.9	10	−0.9	−0.9	4	0.750	6.500	6.000	0.250	0.0
		16	0.0	2.1	17	1.0	−0.9						

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type	Proj (in)
O 24	Endwall	3/4"	GR36	2.00
O 24	Frame	3/4"	GR36	2.00

BUILDING BRACING REACTIONS

Wall Loc	Wall Line	Col Line	± Reactions (k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel Shear (lb/ft) Wind	Seis	Note
L_EW	1								(h)
F_SW	A	2,3	5.0	4.1	1.8	1.5			
R_EW	4	C;D	1.9	1.7	0.7	0.6			
B_SW	F	3,2	5.0	4.1	1.8	1.5			

(h)Rigid frame at endwall

NOTES FOR REACTIONS

1. All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
2. Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
3. Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
4. Building reactions are based on the following building data:

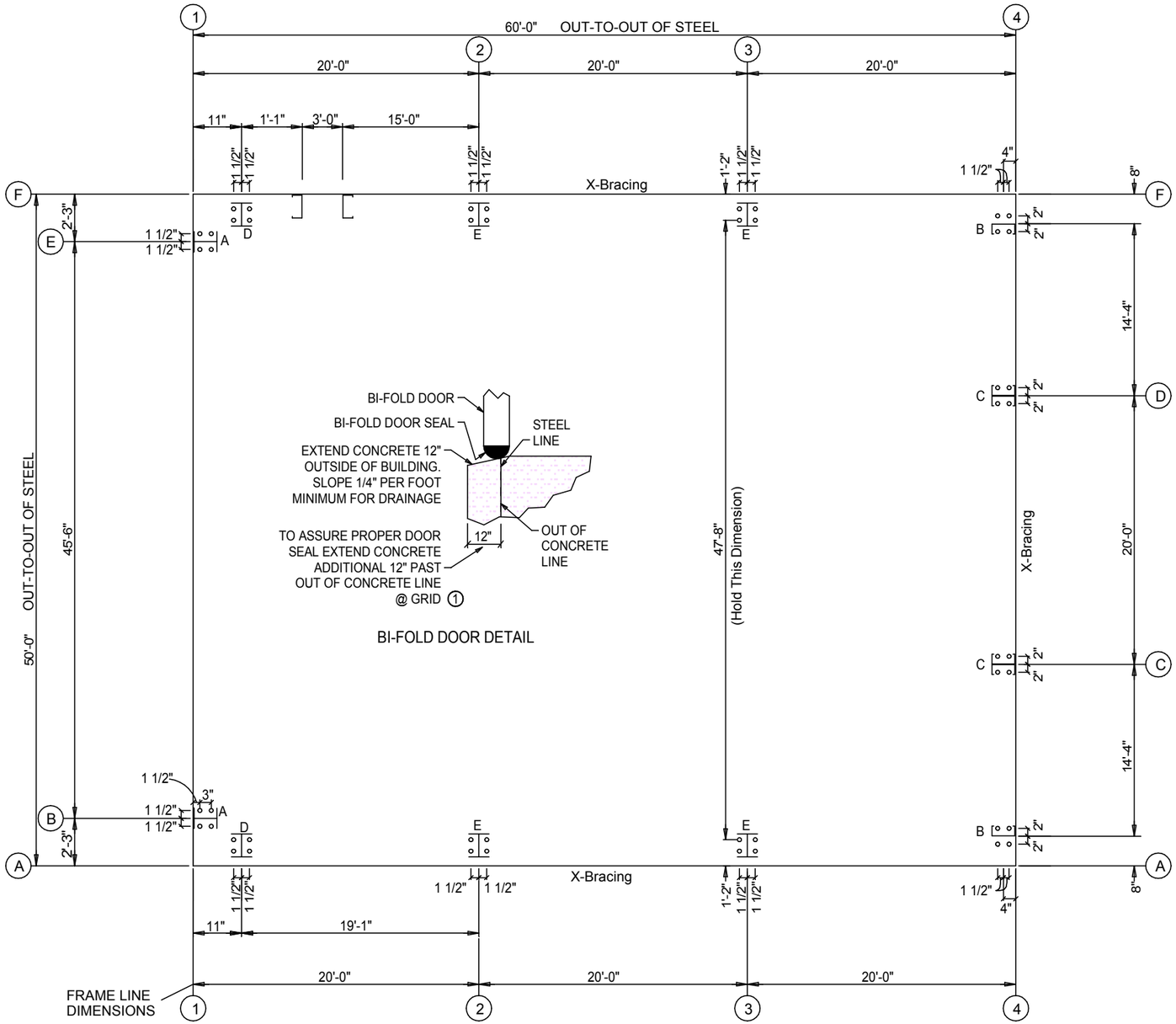
Width (ft)	= 50.0
Length (ft)	= 60.0
Eave Height (ft)	= 18.0/ 18.0
Roof Slope (rise/12)	= 1.0/ 1.0
Dead Load (psf)	= 2.3
Collateral Load (psf)	= 1.0
Live Load (psf)	= 20.0
Ground Snow Load (psf) (Pg)	= 42.85
Roof Snow Load (psf) (Pf)	= 30.0
Wind Speed (mph)	= 115.0
Wind Code	= IBC 12
Exposure	= C
Closed/Open	= C
Importance Wind	= 1.00
Importance Seismic	= 1.00
Seismic Zone	= D
Seismic Coeff (Fa*Ss)	= 0.87

Pf = 0.7 Ce Ct I Pg
Ce = 1
Ct = 1
I = 1
Pg = 42.85 PSF
Pf = 30.0 PSF
UNBALANCED LOAD = 40.5 PSF

5. Loading conditions are:
   1. Dead+Collateral+Snow
   2. Dead+Collateral+Snow+Slide_Snow
   3. 0.6Dead+0.6Wind_Left1
   4. 0.6Dead+0.6Wind_Right1
   5. 0.6Dead+0.6Wind_Long1
   6. 0.6Dead+0.6Wind_Long2
   7. Dead+Collateral+F1UNB_SL_L
   8. Dead+Collateral+F1UNB_SL_R
   9. 0.6Dead+0.6Wind_Right2+0.6Wind_Suction
   10. 0.6Dead+0.6Wind_Pressure+0.6Wind_Long2
   11. 1.08Dead+1.08Collateral−0.7Seismic_Long
   12. 0.6Dead+0.6Wind_Suction+0.6Wind_Long1
   13. 0.6Dead+0.6Wind_Pressure+0.6Wind_Long1
   14. Dead+Collateral+E2UNB_SL_L
   15. 0.6Dead+0.6Wind_Left2+0.6Wind_Suction
   16. Dead+Collateral+E2UNB_SL_R
   17. 0.6Dead+0.6Wind_Suction+0.6Wind_Long2

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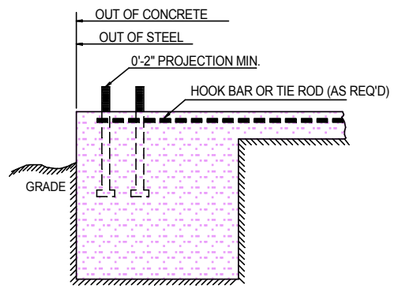
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DATE: 2/5/16		
		DRAWN BY
		DRAWING NUMBER
		OF



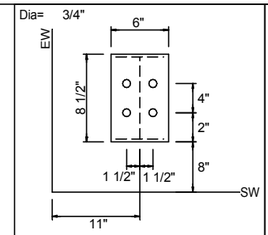
ANCHOR BOLT PLAN

NOTE: All Base Plates @ 100'-0" (U.N.)

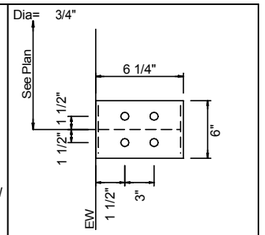
BI-FOLD DOOR DETAIL



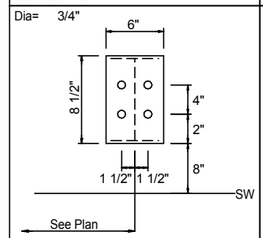
ANCHOR BOLT CONCRETE DETAIL



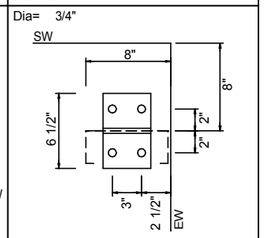
DETAIL D



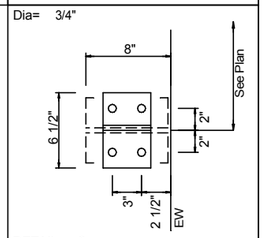
DETAIL A



DETAIL E



DETAIL B



DETAIL C

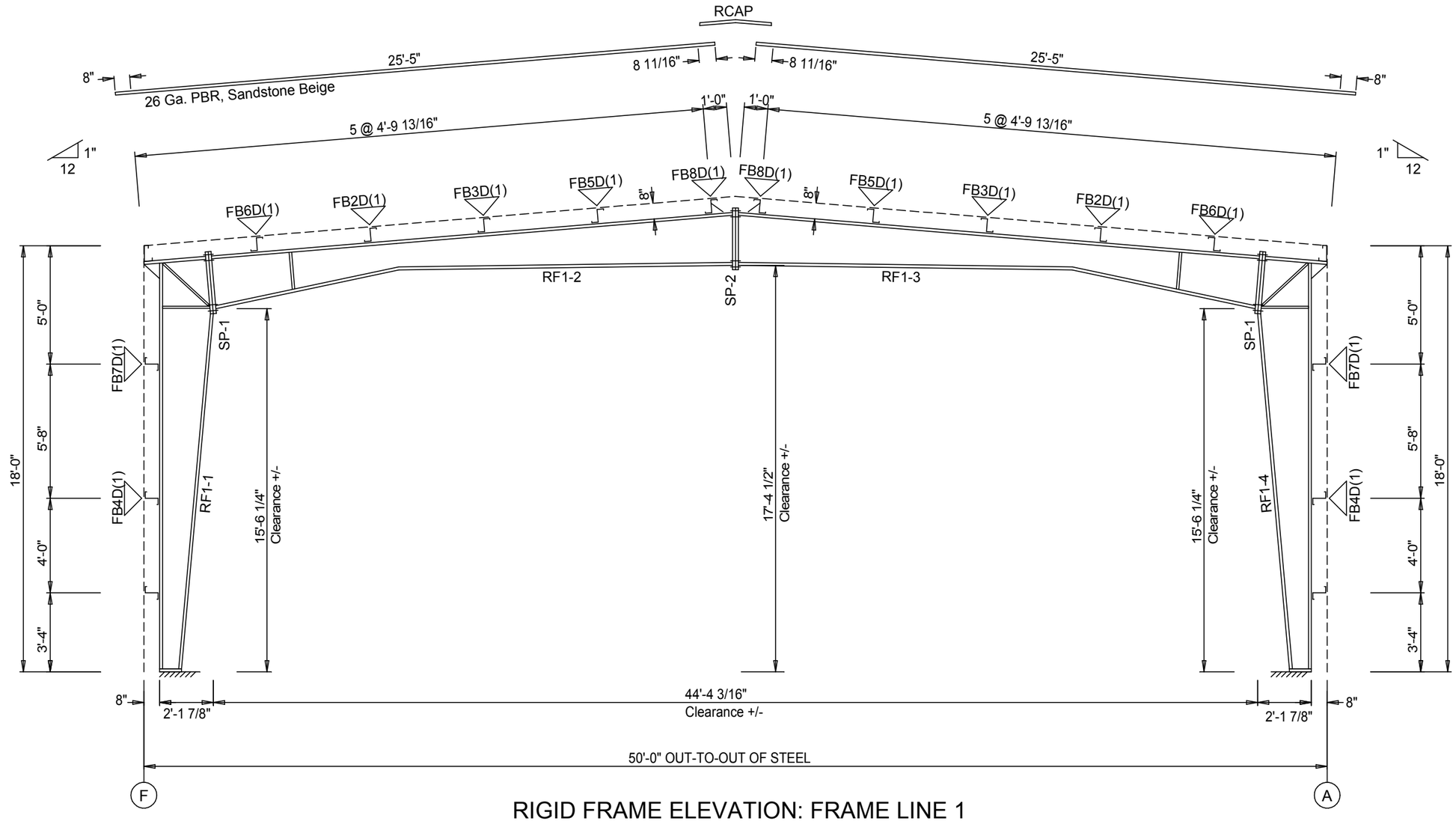
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SCALE:	JOB LOCATION	REVISION
		DRAWN BY CJH
		DRAWING NUMBER OF

SPLICE PLATE & BOLT TABLE									
Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	0	A325	0.750	2.00	6"	1/2"	2'-6 13/16"
SP-2	4	4	0	A325	0.750	2.00	6"	1/2"	2'-6 11/16"

MEMBER TABLE					
Mark	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF1-1, RF1-4	8.0/25.4	0.150	209.8	6 x 1/4" x 207.9 6 x 1/4" x 31.5	6 x 1/4" x 183.3
RF1-2	24.0/12.0	0.150	96.0	6 x 1/4" x 268.1	6 x 1/4" x 96.7
	12.0/23.9	0.150	172.0		6 x 1/4" x 170.4
RF1-3	23.9/12.0	0.150	172.0	6 x 1/4" x 268.1	6 x 1/4" x 170.4
	12.0/24.0	0.150	96.0		6 x 1/4" x 96.7

FLANGE BRACES: Both Sides(U.N.)
FBxxD(1)
D - L2x2x14



RIGID FRAME ELEVATION: FRAME LINE 1

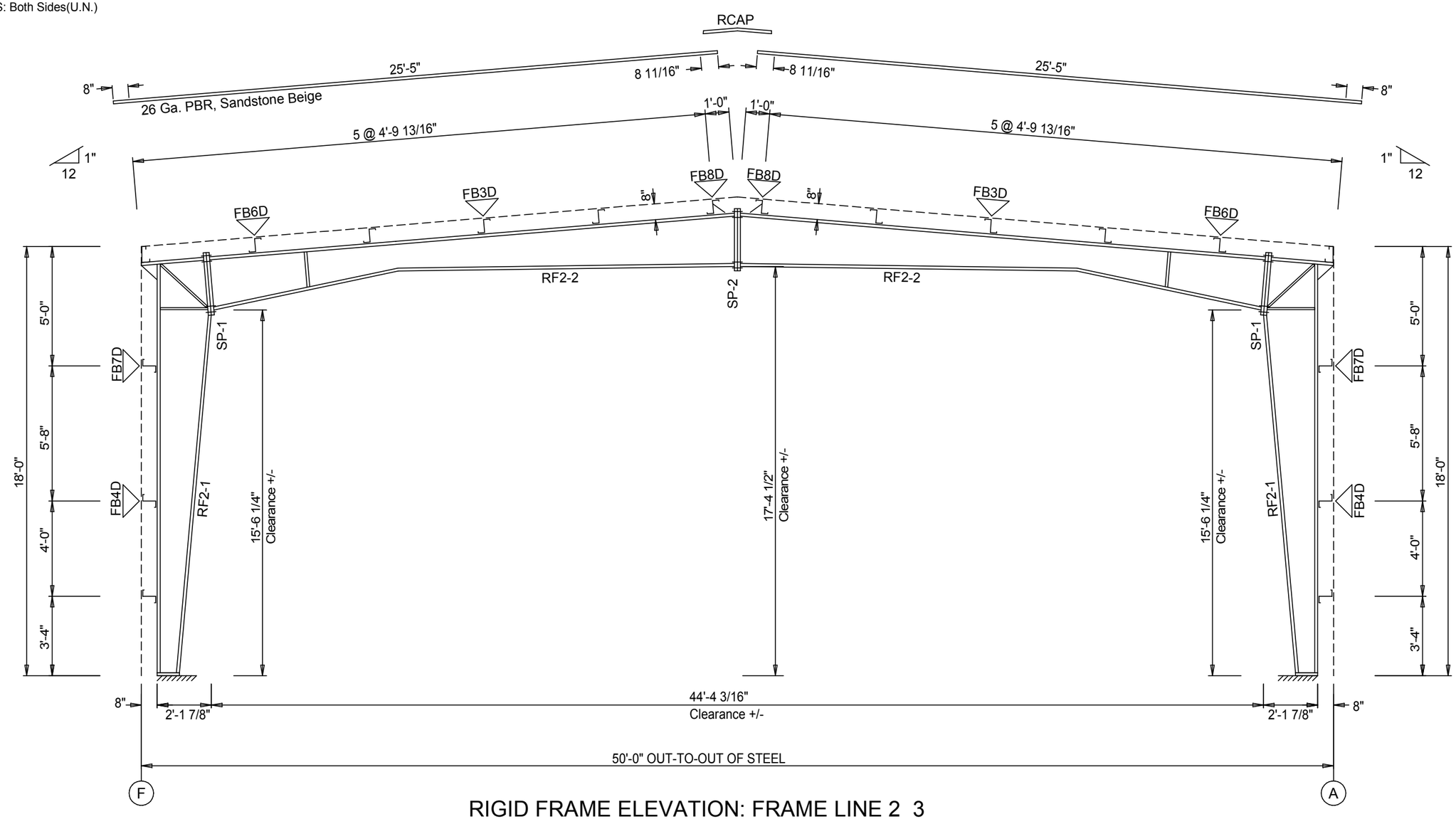
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SCALE:	JOB LOCATION	REVISION
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		DRAWING NUMBER
		OF

SPLICE PLATE & BOLT TABLE									
Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	0	A325	0.750	2.00	6"	1/2"	2'-6 13/16"
SP-2	4	4	0	A325	0.750	2.00	6"	1/2"	2'-6 11/16"

MEMBER TABLE					
Mark	Web Depth Start/End	Web Plate Thick	Web Plate Length	Outside Flange W x Thk x Length	Inside Flange W x Thk x Length
RF2-1	8.0/25.4	0.150	209.8	6 x 1/4" x 207.9 6 x 1/4" x 31.5	6 x 1/4" x 183.3
RF2-2	24.0/12.0 12.0/23.9	0.150 0.150	96.0 172.0	6 x 1/4" x 268.1	6 x 1/4" x 96.7 6 x 1/4" x 170.4

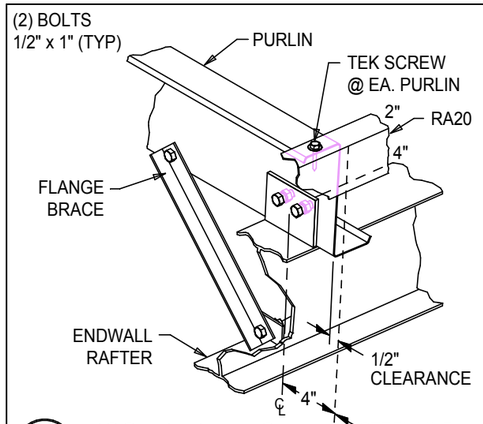
FLANGE BRACES: Both Sides(U.N.)
FBxxD(1)
D - L2x2x14



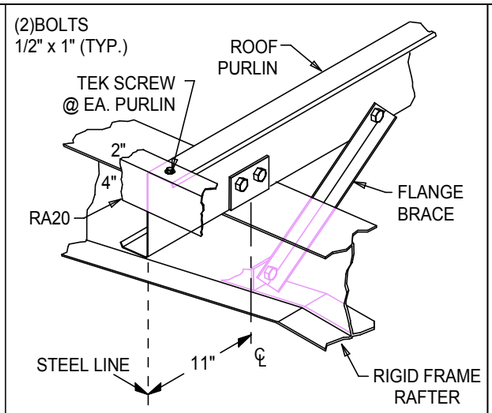
RIGID FRAME ELEVATION: FRAME LINE 2 3

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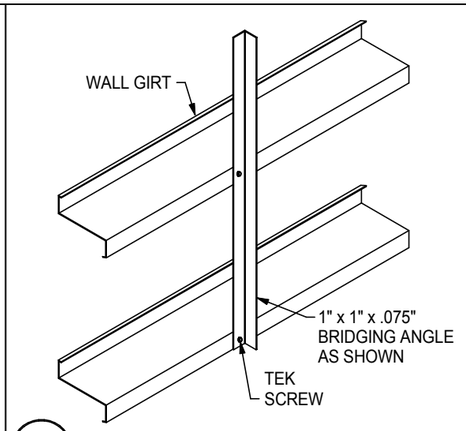
SCALE:	JOB LOCATION	REVISION
		DRAWN BY
		DRAWING NUMBER
		OF



A3 ROOF PURLIN TO ENDWALL RAFTER

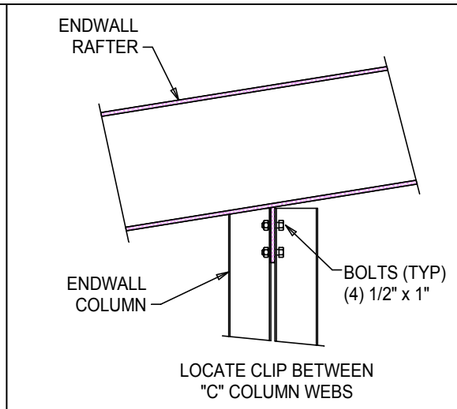


A9 ROOF PURLIN TO ENDWALL RIGID FRAME

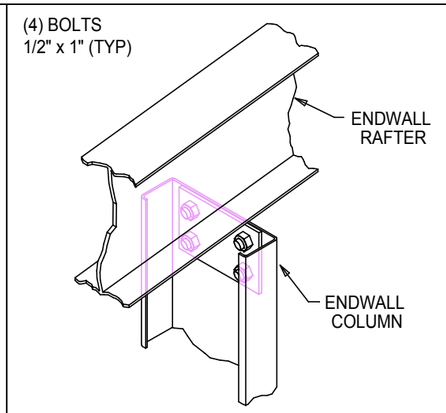


B2 WALL GIRT BRIDGING

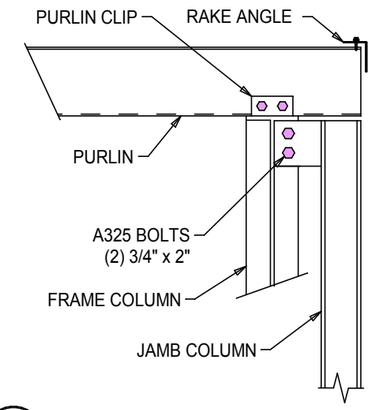
3/30/09 WD



B4 ENDWALL RAFTER TO COLUMN

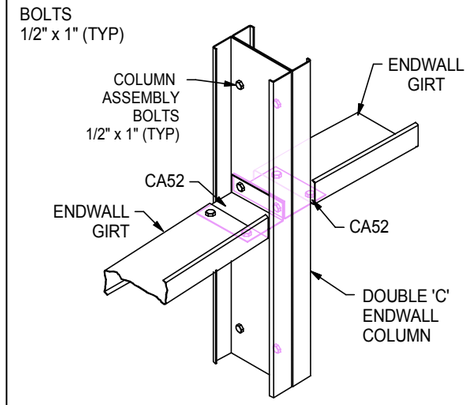


B7 ENDWALL COLUMN TO ENDWALL RAFTER

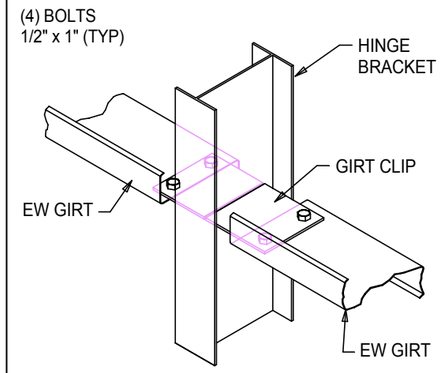


B35 JAMB COLUMN TO FRAME COLUMN (W / 11" SETBACK)

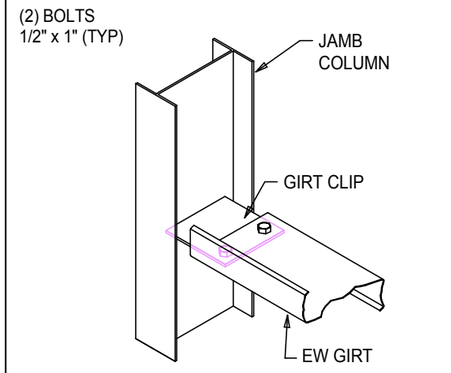
4/2/09 WD



C2 DOUBLE 'C' ENDWALL COLUMN TO GIRTS

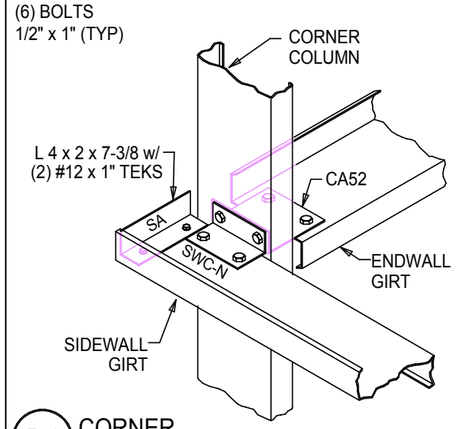


C6 HINGE BRACKET TO ENDWALL GIRT



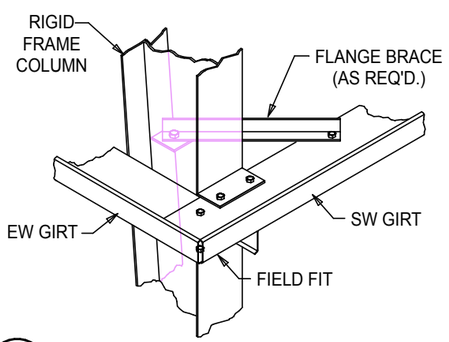
C7 JAMB COLUMN TO ENDWALL GIRT

3/26/09 JMS

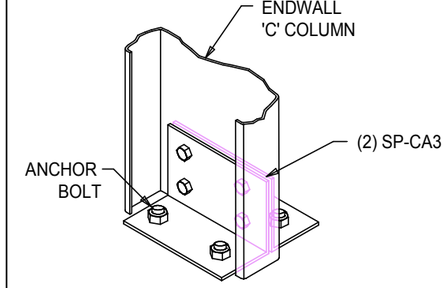


D1 CORNER COLUMN TO WALL GIRTS

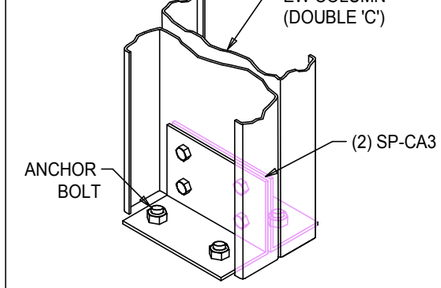
4/ 9/09 WD



D16 CORNER COLUMN TO GIRTS



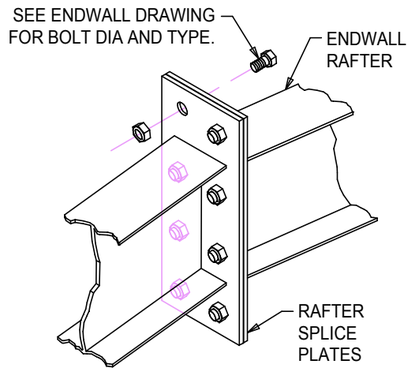
E1 BASE PLATE FOR EW COLUMN



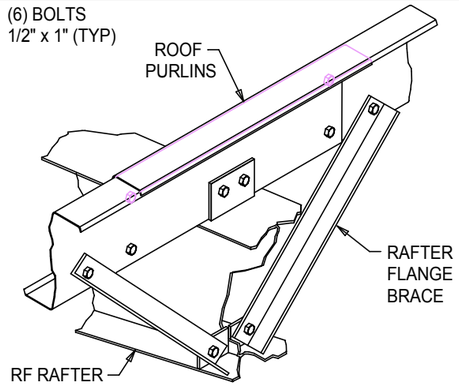
E2 BASE PLATE FOR EW COLUMN

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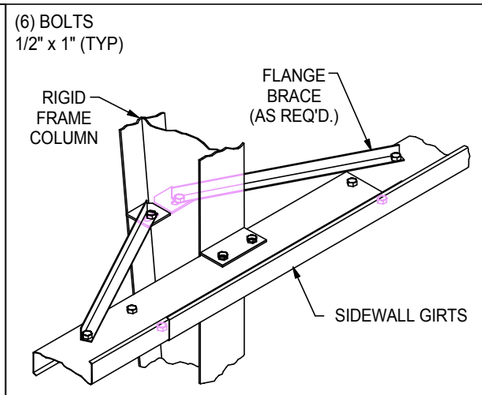
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		DRAWING NUMBER
		OF



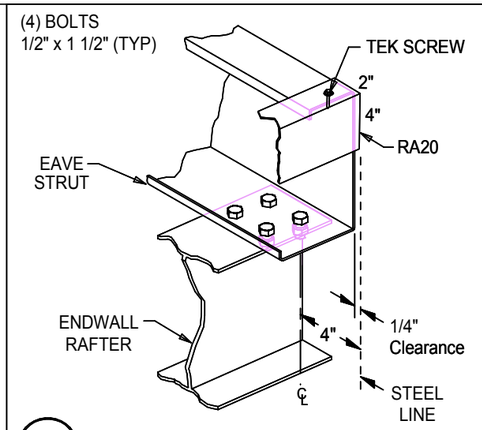
F12 ENDWALL RAFTER SPLICE AT RIDGE



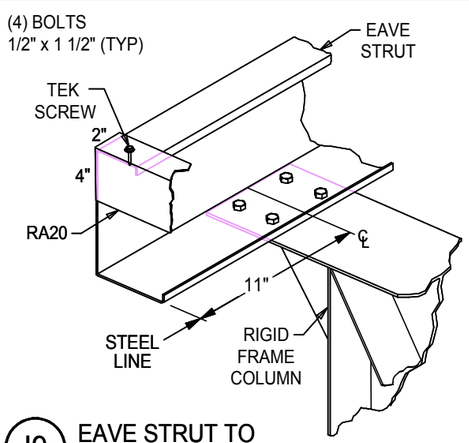
G1 ROOF PURLIN TO INTERIOR RIGID FRAME RAFTER



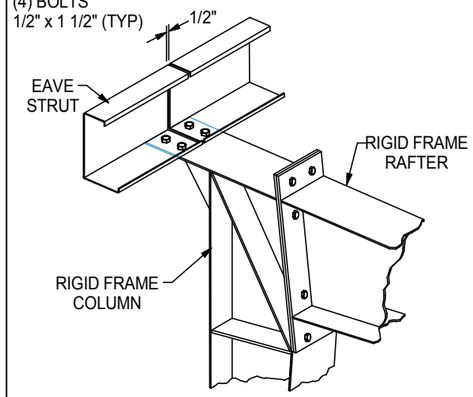
H1 SIDEWALL GIRT TO INTERIOR FRAME COLUMN



I8 EAVE STRUT TO ENDWALL RAFTER

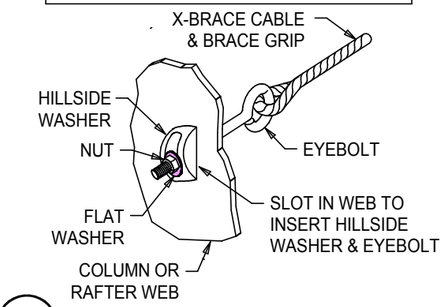


I9 EAVE STRUT TO RIGID FRAME COLUMN

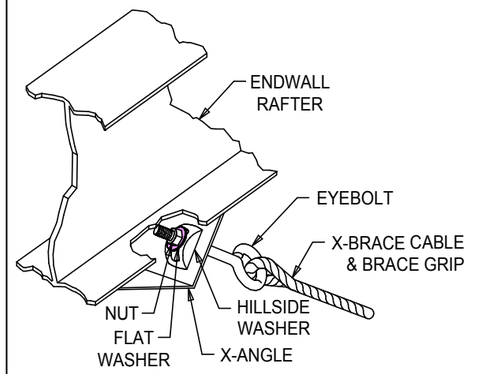


J2 EAVE STRUT TO RIGID FRAME @ INTERIOR BAYS

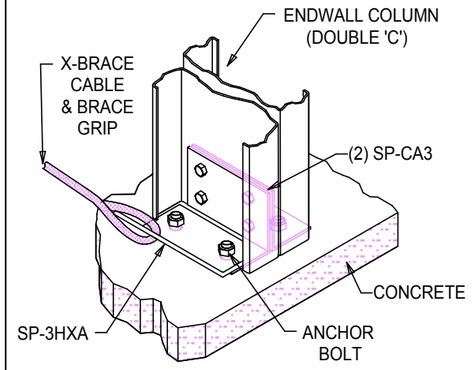
5/8"	E.H.S. CABLE WITH	1 1/4" EYEBOLT
1/2"	E.H.S. CABLE WITH	7/8" EYEBOLT
3/8"	E.H.S. CABLE WITH	3/4" EYEBOLT
5/16"	E.H.S. CABLE WITH	5/8" EYEBOLT
1/4"	E.H.S. CABLE WITH	1/2" EYEBOLT



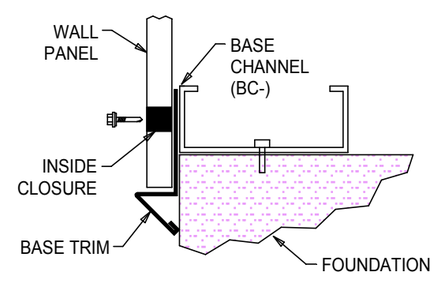
Q2 DIAGONAL CABLE, EYEBOLT END



Q4 DIAGONAL CABLE, EYEBOLT END



Q5 SP-3HXA & (2) SP-CA3 @ ENDWALL COLUMN

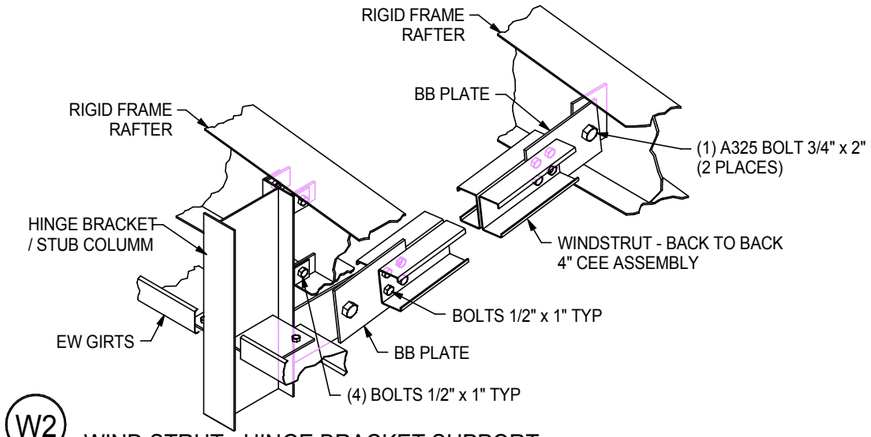


CONCRETE FOUNDATION BY OTHERS, MUST BE ADEQUATE TO SAFELY RESIST THE LOADS IN THE REACTION REPORT.

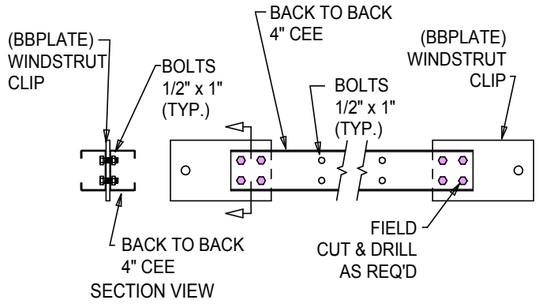
T2 SECTION THRU WALL PANEL AND CONCRETE FOUNDATION

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W2 WIND STRUT - HINGE BRACKET SUPPORT



WINDSTRUT ASSEMBLY DETAIL

1. FIELD CUT X-BRACE THRU GIRTS (AS REQ'D).
2. FIELD CUT X-BRACE TO LENGTH (AS REQ'D).
3. FIELD FIT AND DRILL GIRTS AT CORNER COLUMN (AS REQ'D).
4. FIELD LOCATE WALK DOORS, WINDOWS, AND LOUVERS (AS REQ'D).
5. FIELD FIT WINDSTRUT (AS REQ'D).
6. FIELD CUT 3'-4" ELEV. GIRT AND TEK TO 8 1/4" WALK DOOR JAMB.
7. FIELD FIT FLANGE BRACES AS REQ'D.
8. LAP BRIDGE ANGLE 1", USE (2) 12-14 x 1" TEKS.
9. LAP RAKE ANGLE 1", USE (2) 12-14 x 1" TEKS.

Y1 FIELD NOTES

FORMAT IS BDDxWTHH

B = WELDED MEMBER TYPE.
DD = MEMBER DEPTH IN INCHES.
W = FLANGE WIDTH IN INCHES.
(6=6", 8=8", 0=10", 2=12")
T = FLANGE THICKNESS IN 1/16" INCH UNITS.
(3=3/16", 4=1/4", 5=5/16" 6=3/8", 8=1/2", 0=5/8", 2=3/4")
HH = WEB THICKNESS IN DECIMAL INCH UNITS.
(14=.1495, 17=.1793, 25=.2500, 31=.3125)

Y2 BUILT-UP NOTES

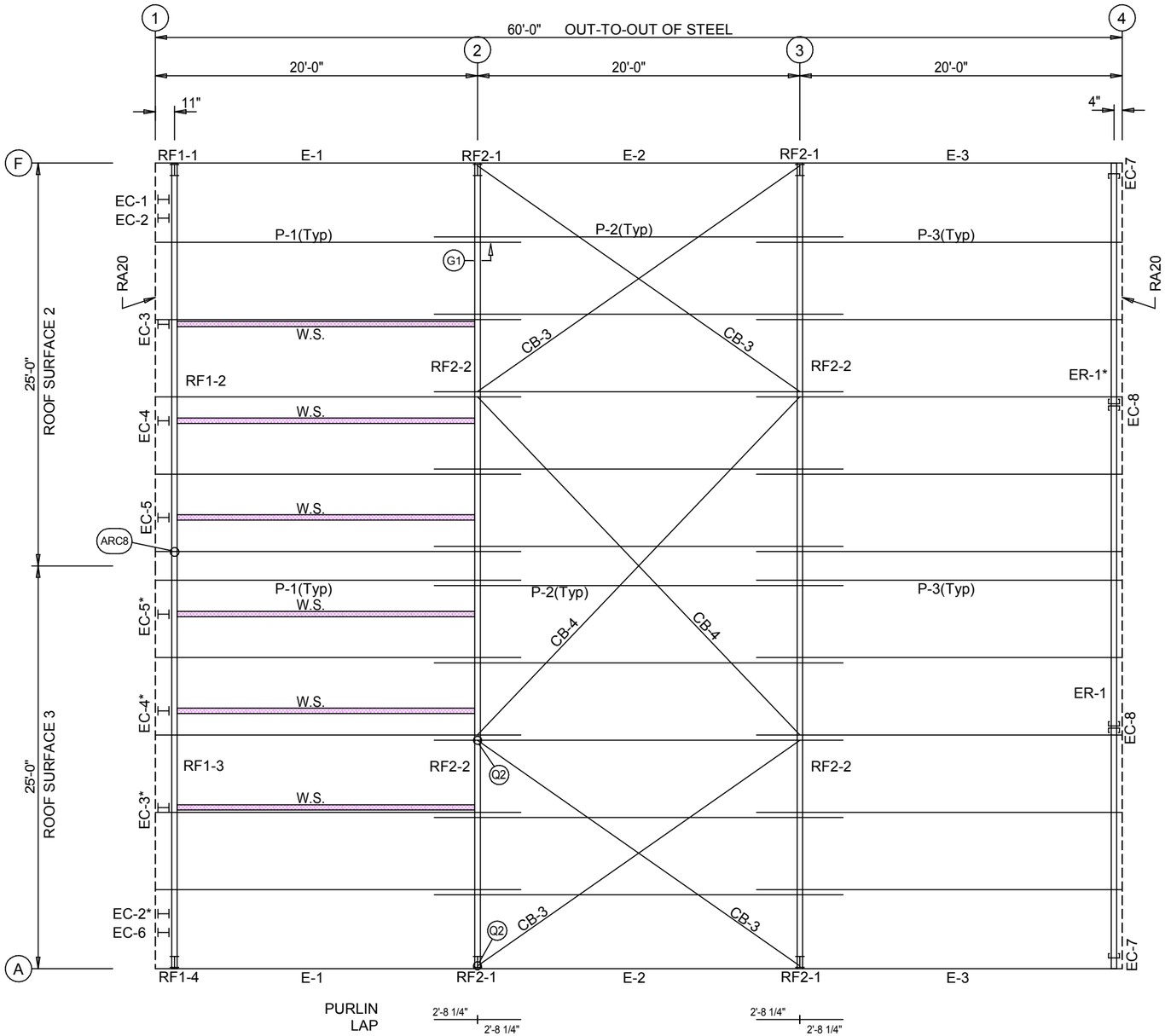
ARC8 ANTI-ROLL CLIP DETAIL

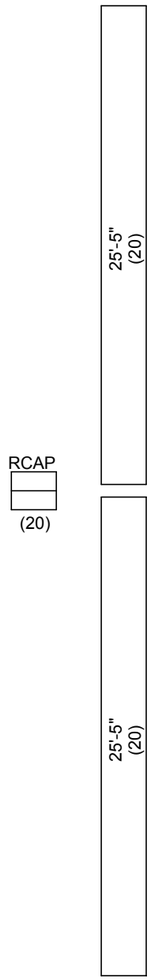
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MEMBER TABLE ROOF PLAN		
MARK	PART	LENGTH
P-1	8x25Z13	22'-8 1/4"
P-2	8x25Z15	25'-4 1/2"
P-3	8x25Z13	22'-7 3/4"
E-1	80x5E14	19'-11 3/4"
E-2	80x5E14	19'-11 1/2"
E-3	80x5E14	19'-11 1/2"
CB-3	1/4EHS	23'-0"
CB-4	1/4EHS	28'-0"

INSTALL ROOF FLANGE BRACE AS PER RIGID FRAME CROSS SECTION





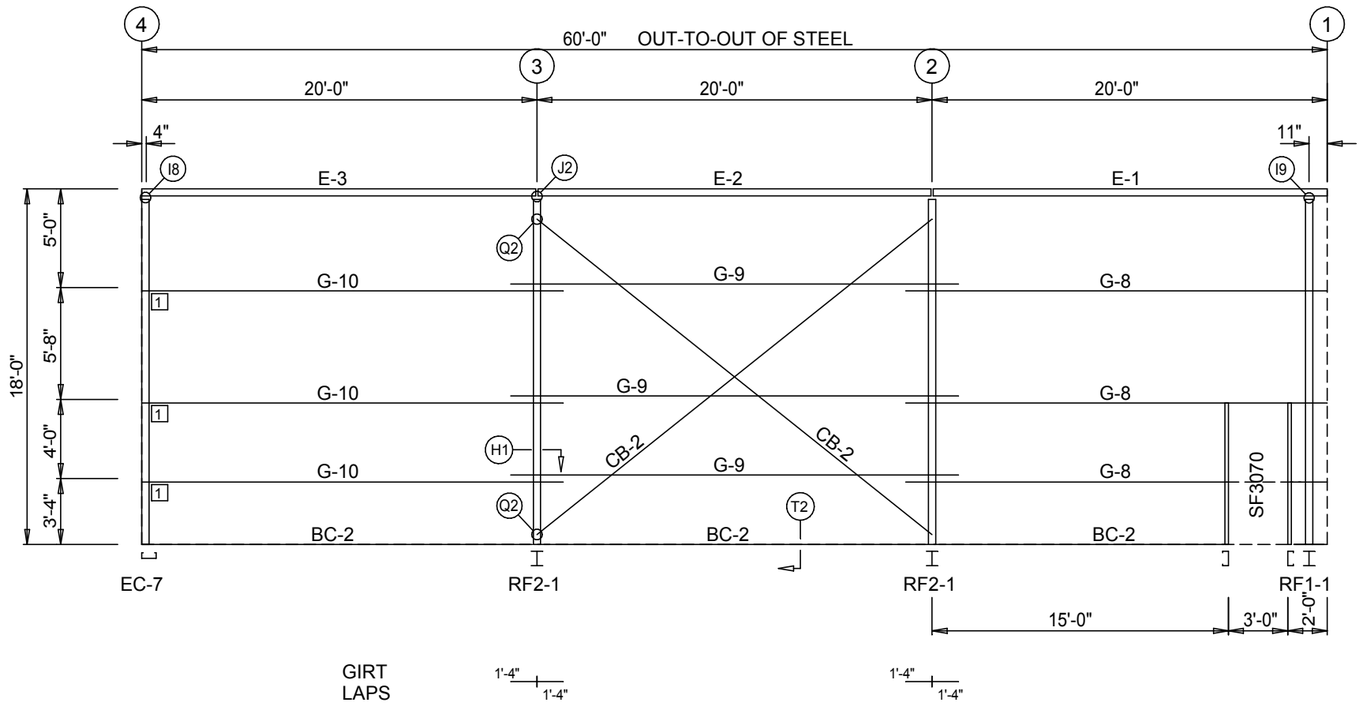
ROOF SHEETING

PANELS: 26 Ga. PBR
Sandstone Beige

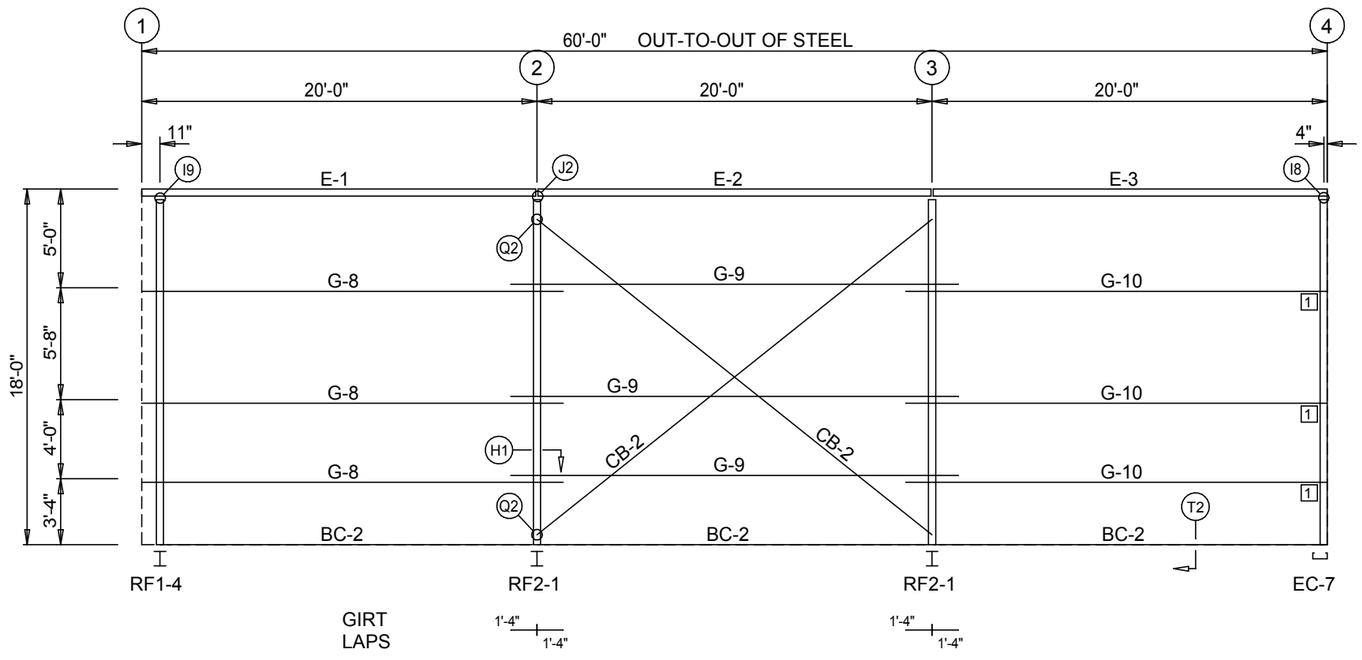
ROOF FRAMING PLAN

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SIDEWALL FRAMING: GRID F



SIDEWALL FRAMING: GRID A

MEMBER TABLE
GRID A & F

MARK	PART	LENGTH
E-1	80x5E14	19'-11 3/4"
E-2	80x5E14	19'-11 1/2"
E-3	80x5E14	19'-11 1/2"
G-8	8x25Z16	21'-4"
G-9	8x25Z16	22'-8"
G-10	8x25Z16	21'-4"
BC-2	8x25C16	20'-0"
CB-2	5/16EHS	25'-0"

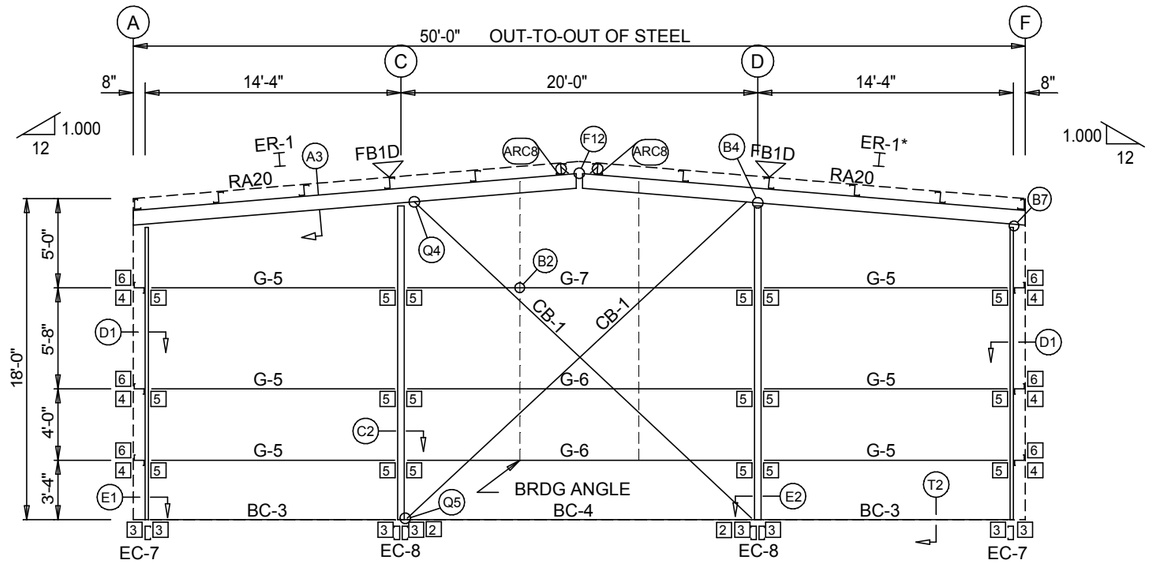
CONNECTION PLATES
GRID A & F

☐ ID	MARK/PART
1	SWC-N

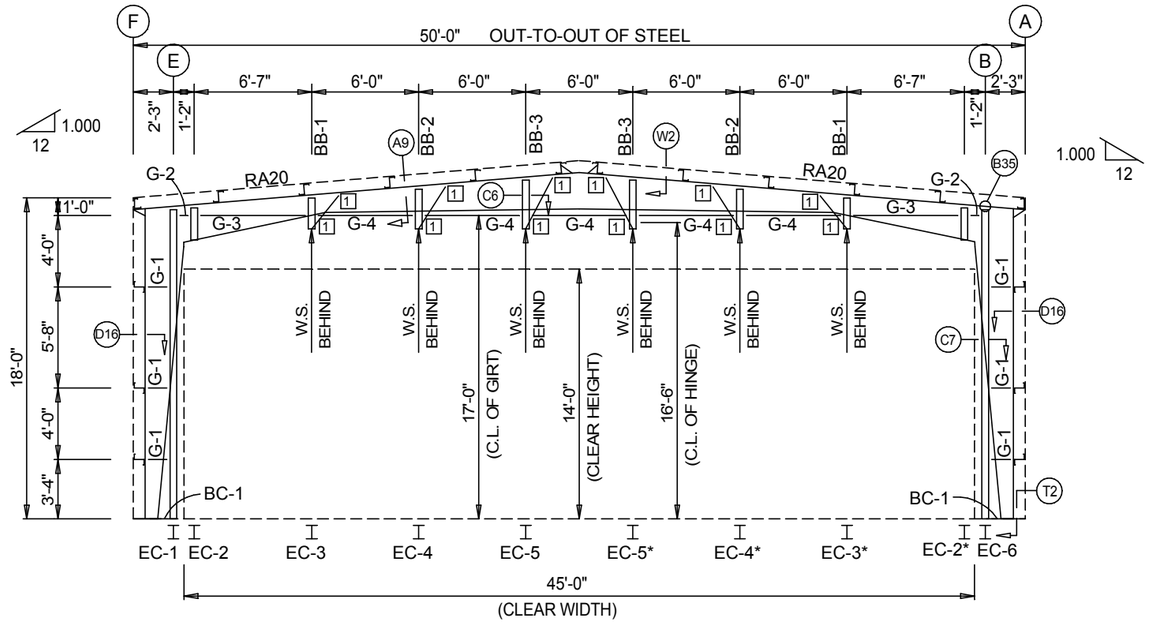
INSTALL SIDEWALL FLANGE BRACE
AS PER RIGID FRAME CROSS SECTION

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		OF



ENDWALL FRAMING: GRID 4



ENDWALL FRAMING: GRID 1

BOLT TABLE GRID 1 & 4				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-1/ER-1*	8	A325	5/8"	1 3/4"
EC-1/FRAME	2	A325	3/4"	2"
EC-2/FRAME	4	GR_5	1/2"	1"
EC-3/FRAME	4	GR_5	1/2"	1"
EC-4/FRAME	4	GR_5	1/2"	1"
EC-5/FRAME	4	GR_5	1/2"	1"
EC-6/FRAME	2	A325	3/4"	2"
Cor_Column/Raf	4	GR_5	1/2"	1"
EC-8/ER-1	4	GR_5	1/2"	1"
EC-8/ER-1*	4	GR_5	1/2"	1"
Back Braces	2	A325	3/4"	2"

MEMBER TABLE GRID 1 & 4		
MARK	PART	LENGTH
BB-1	D4x5C16	18'-0 1/16"
BB-2	D4x5C16	18'-0 9/16"
BB-3	D4x5C16	18'-1 1/4"
EC-1	W6x20	17'-6 1/8"
EC-2	W8x10	1'-11 3/4"
EC-3	W8x10	1'-11"
EC-4	W8x10	2'-5"
EC-5	W8x10	2'-11"
EC-6	W6x20	17'-6 1/8"
EC-7	8x25C15	16'-8 3/4"
EC-8	D8x5C14	17'-11 1/16"
ER-1	W8x10	25'-1 1/16"
ER-1*	W8x10	25'-1 1/16"
G-1	8x25Z16	1'-11"
G-2	8x25Z16	0'-6"
G-3	8x25Z16	5'-11"
G-4	8x25Z16	5'-4"
G-5	8x25Z16	13'-8"
G-6	8x25Z16	19'-4"
G-7	8x25Z14	19'-4"
BC-1	8x25C16	1'-11"
BC-3	8x25C16	13'-8"
BC-4	8x25C16	19'-4"
CB-1	1/4EHS	25'-0"

FLANGE BRACE TABLE GRID 1 & 4		
▽ID	MARK	LENGTH
1	FB1D	1'-4 3/4"

CONNECTION PLATES GRID 1 & 4	
☐ ID	MARK/PART
1	BBPLATE
2	SP-3HXA
3	SP-CA3
4	SWC-N
5	CA52
6	SA

INSTALL ENDWALL FLANGE BRACE

BIFOLD DOOR NOTE:

1.) ADJUST BIFOLD DOOR JAMB WHEEL TO APPROXIMATE CENTER OF JAMB COLUMN (1/2" LESS).

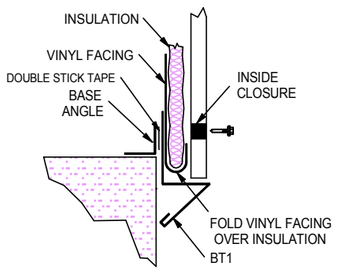
2.) FIELD DRILL & BOLT HINGE TO HB

3.) FIELD WELD HINGE TO HB

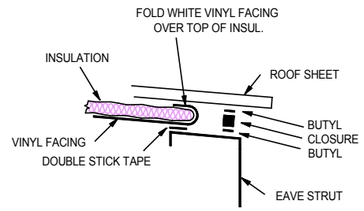
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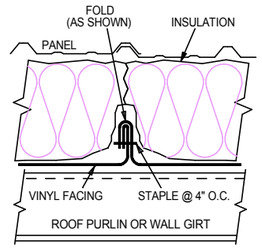
INSULATION TABLE ROOF PLAN = R19 - 6"				
ROLL	QUAN	MARK	WIDTH	LENGTH
1	10	ROOF	6'-0"	51'-6"



INSULATION DETAIL



INSULATION DETAIL
@ EAVE



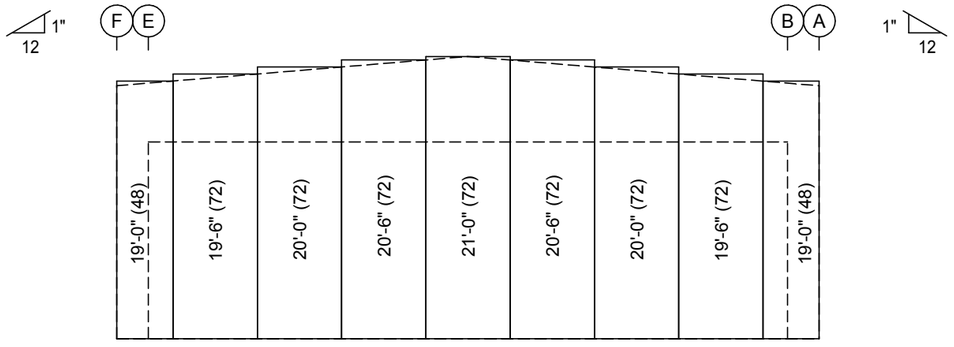
INSULATION DETAIL
@ SIDE LAP

ENDWALL INSULATION: GRID 4

INSULATION: 4.00" Thick VR

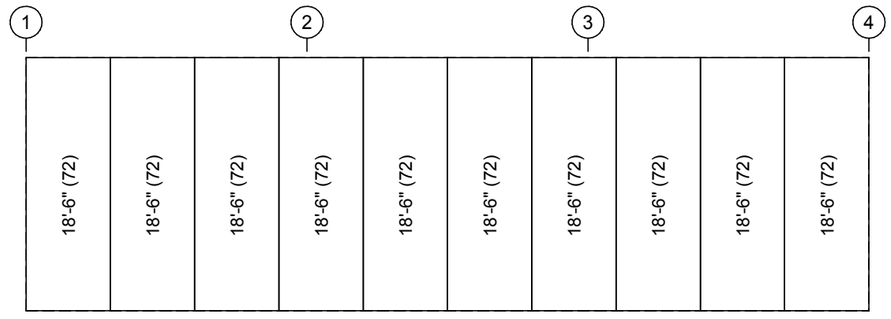
SIDEWALL INSULATION: GRID F

INSULATION: 4.00" Thick VR



ENDWALL INSULATION: GRID 1

INSULATION: 4.00" Thick VR

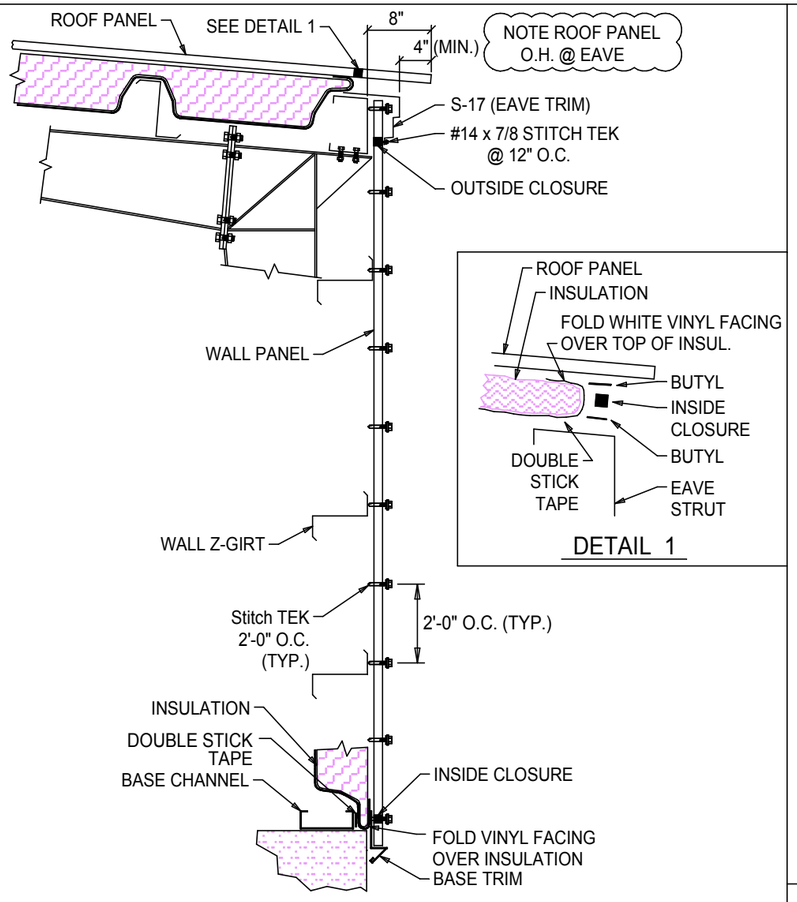


SIDEWALL INSULATION: GRID A

INSULATION: 4.00" Thick VR

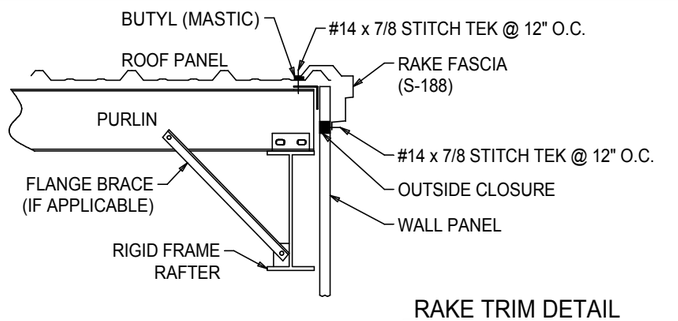
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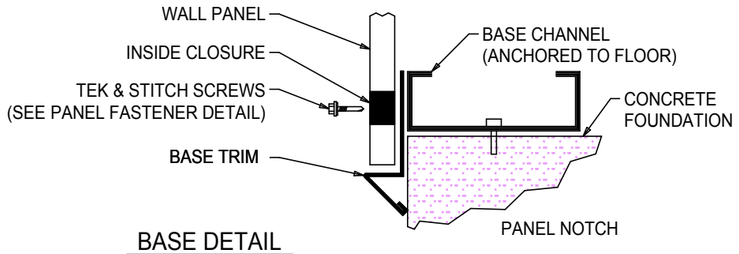


S-17 EAVE TRIM DETAIL & WALL STITCH TEK LAYOUT

DETAIL 1



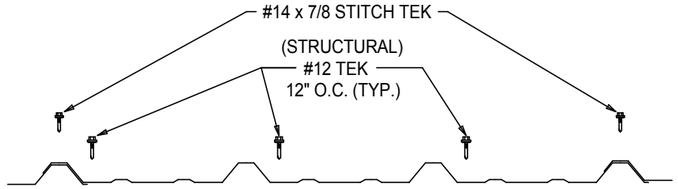
RAKE TRIM DETAIL



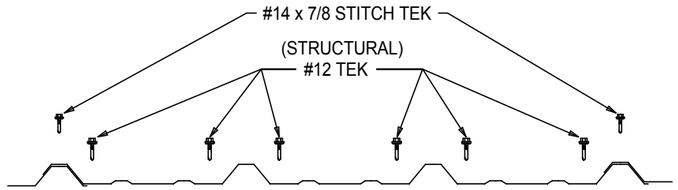
BASE DETAIL

TYPICAL TEK SPACING FOR HIGH RIB PANELS

NOTE: ALIGN ROOF PANEL HI-RIBS WITH WALL PANEL HI-RIBS.

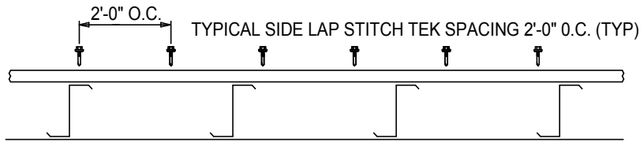


I. TYPICAL SPACING AT INTERMEDIATE PURLINS & GIRTS.

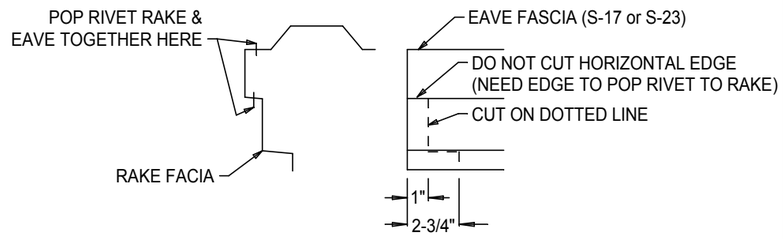


II. TYPICAL SPACING AT PANEL ENDS

1) TYPICAL SPACING AT EAVE STRUT.
2) TYPICAL SPACING AT RIDGE CAP.
3) TYPICAL SPACING AT BASE ANGLE.
4) TYPICAL SPACING AT RAKE ANGLE.
5) TYPICAL SPACING AT PANEL LAP.



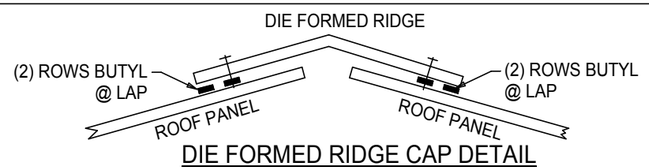
III. TYPICAL STITCH TEK SPACING (SIDE LAP ONLY)



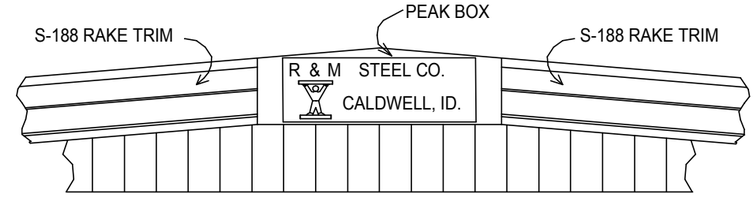
MITER DETAIL: EAVE-TO-RAKE



ROOF PANEL MASTIC DETAIL
@ ROOF PANELS ONLY



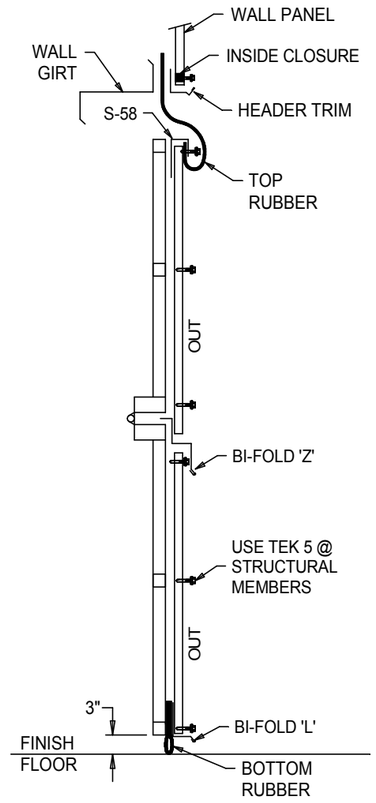
DIE FORMED RIDGE CAP DETAIL



PEAK BOX DETAIL

NOTE:
1) MITER S-188 RAKE TRIM @ PEAK & SEAL.
2) INSTALL PEAKBOX & SEAL.
3) MITER S-188 AND S-17 @ CORNERS & SEAL.

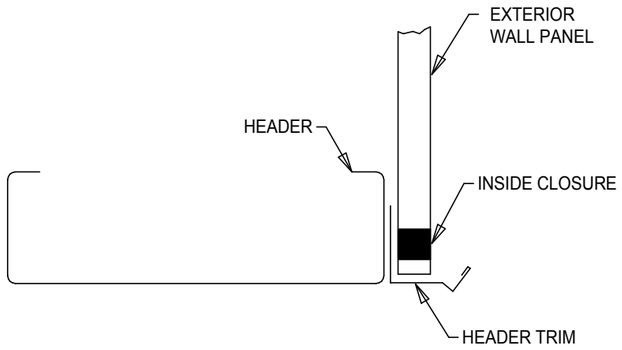
NOTE: FIELD PAINT EXPOSED BI-FOLD DOOR STRUCTURAL STEEL (BY OTHERS).



END VIEW
BI-FOLD DOOR TRIM LAYOUT

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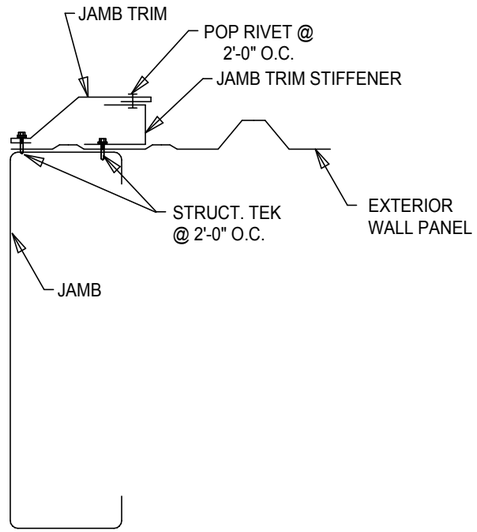
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		DRAWING NUMBER
		OF



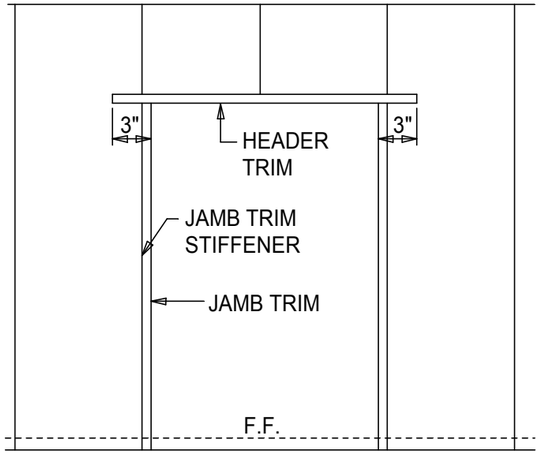
NOTE: FIELD NOTCH PANEL AND SLIDE HEADER TRIM BEHIND PANEL AS REQ.

CAULKING (BY OTHERS)

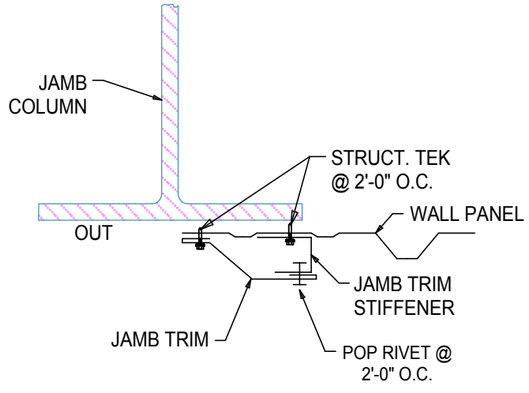
HEADER TRIM DETAIL



JAMB TRIM DETAIL



FRAMED OPENING DETAIL FOR OVERHEAD DOOR OR WALK DOOR

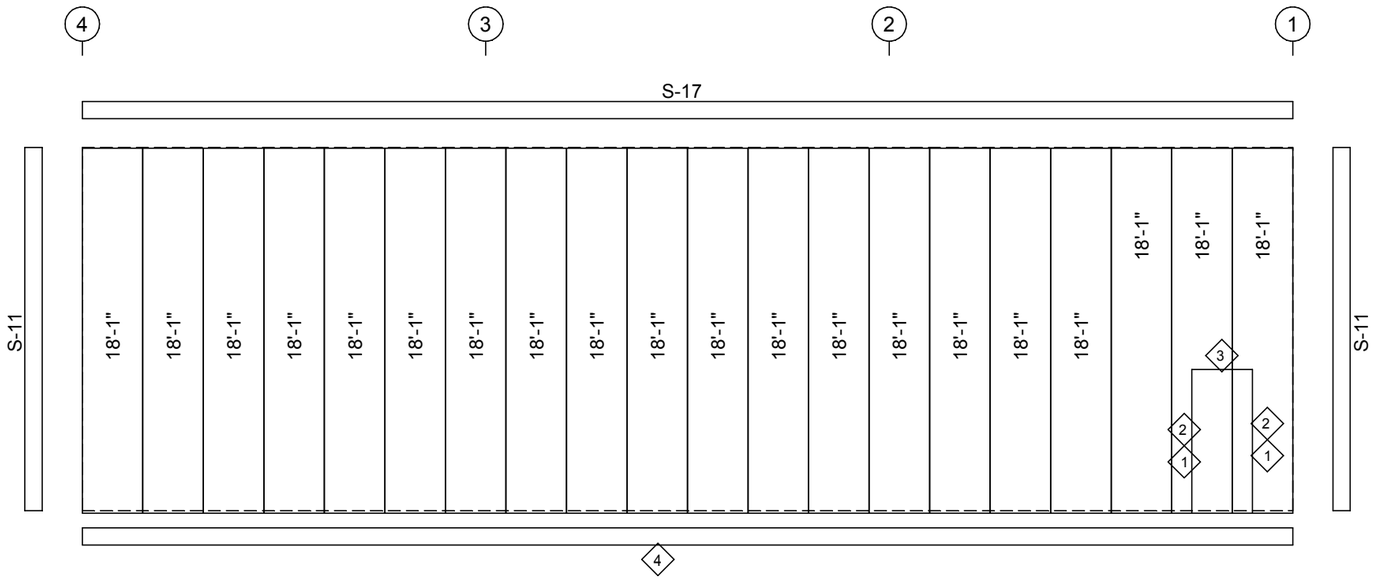


TRIM DETAIL @ JAMB COLUMN FOR BI-FOLD DOOR OPENING

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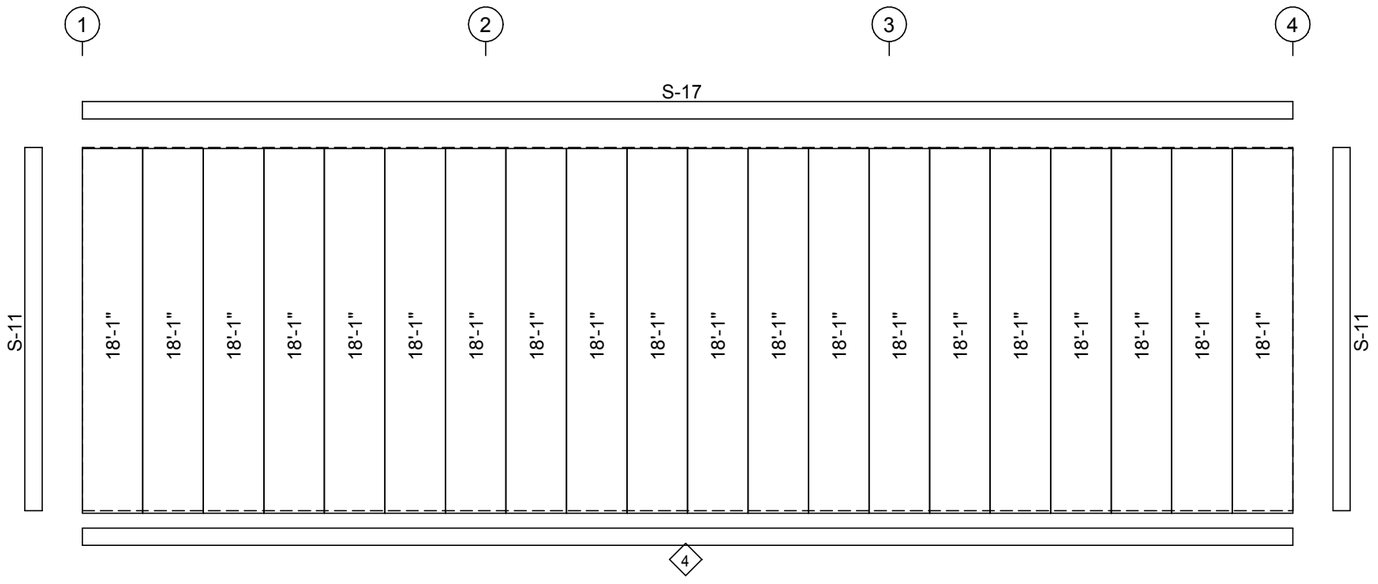
SCALE:	JOB LOCATION	REVISION
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		DRAWING NUMBER
		OF

TRIM TABLE FRAME LINE		A & F
◇ID	MARK	LENGTH
	S-17	31'-6"
	S-11	18'-0"
4	BT1	31'-6"
1	JMBTRM	7'-4"
2	JMBSTF	7'-4"
3	HEDTRM	4'-0"



SIDEWALL SHEETING: GRID F

PANELS: 26 Ga. PBR - Sandstone Beige



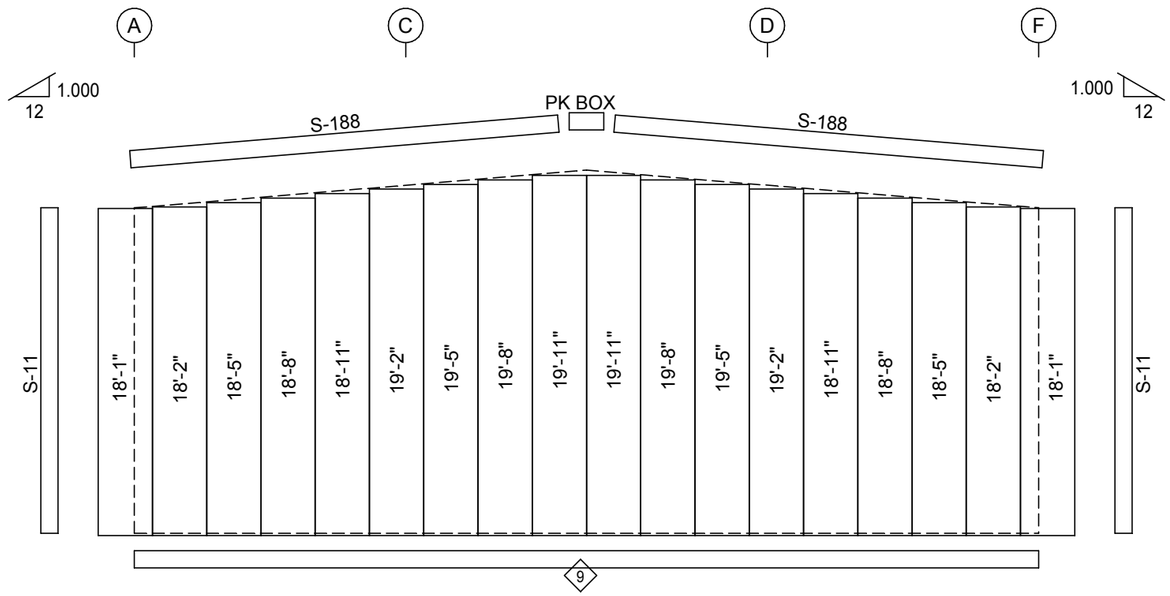
SIDEWALL SHEETING: GRID A

PANELS: 26 Ga. PBR - Sandstone Beige

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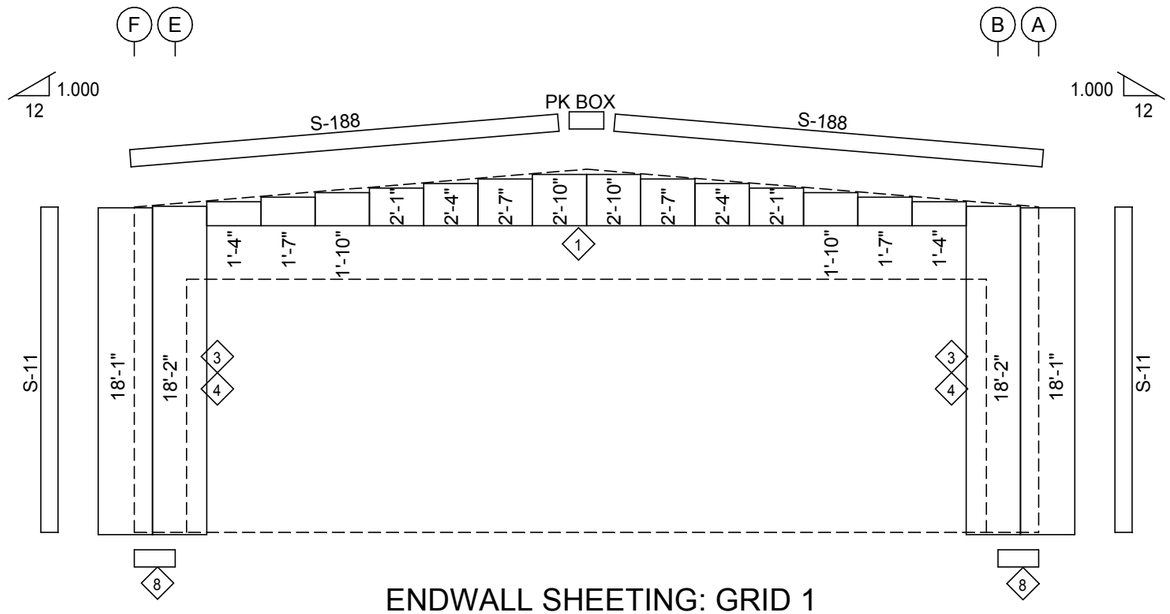
SCALE:	JOB LOCATION	REVISION
		DRAWN BY
		DRAWING NUMBER
		OF

TRIM TABLE FRAME LINE		1 & 4
◇ID	MARK	LENGTH
	PK BOX	2'-6"
	S-188	26'-6"
1	HEDTRM	24'-0"
2	S-58	8'-6"
3	JMBSTF	17'-6"
4	JMBTRM	17'-6"
5	BI-FOLD 'Z'	24'-0"
6	S-58	24'-0"
7	BI-FOLD 'L'	24'-0"
	S-11	18'-0"
8	BT1	4'-6"
9	BT1	26'-6"



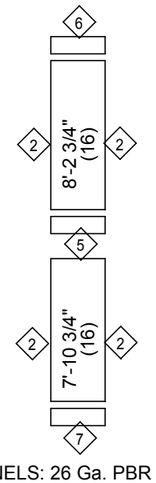
ENDWALL SHEETING: GRID 4

PANELS: 26 Ga. PBR - Sandstone Beige



ENDWALL SHEETING: GRID 1

PANELS: 26 Ga. PBR - Sandstone Beige



PANELS: 26 Ga. PBR
Sandstone Beige

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